

Ques: 1,4,5,6,7

NAME:

ROLL NO:

FST-2 (PAPER-1)

DATE:

Section - A

Q-1-a) Good brick earth contains following ingredients:

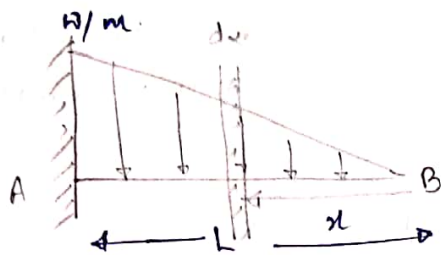
- 1) Alumina (30-40%) : A brick earth must contain alumina as it helps in bricks during moulding.
- 2) Silica (50-60%) : It imparts strength and roundness to the bricks and prevents the bricks from cracking and shrinkage.
- 3) lime (4-5%) : It also imparts strength and roundness to the bricks but if in excess leads to melting of bricks.
- 4) Iron oxide (5-6%) : Imparts reddish brown tinge to the bricks and also imparts strength.
- 5) Magnesia (<1%) : Presence of magnesia in small percentage helps the bricks to gain uniform shape and size.

Harmful Ingredients of Brick Earth:

- 1) Stones and pebbles:
- 2) Organic and vegetative Matter.
- 3) Iron pyrites:
- 4) Presence of soluble salts:

3) Limestone:

Q-1-b)



Consider a strip of ' dx ' @ x from free end.

$$\therefore \text{load @ } x = \left(\frac{w}{L} \times x \right)$$

$$\therefore M_x = EI \frac{d^2y}{dx^2} = \frac{1}{2} \times x \times \frac{w}{L} \times \left(\frac{x}{3} \right)$$

$$\Rightarrow EI \frac{d^2y}{dx^2} = \frac{wx^3}{6L}$$

Int wrt x both sides:

$$EI \left(\frac{dy}{dx} \right) = \frac{wx^4}{24L} + C_1$$

Again on intⁿ:

$$EI(y) = \frac{wx^5}{120L} + C_1x + C_2$$

Putting boundary conditions we get:

$$\text{@ } x=L, \left(\frac{dy}{dx} = 0 \right)$$

$$\Rightarrow 0 = \frac{wL^4}{24L} + C_1 \quad \Rightarrow C_1 = \left(-\frac{wL^3}{24} \right)$$

$$\text{@ } x=L, y=0$$

$$\Rightarrow 0 = \frac{wL^5}{120L} - \frac{wL^4}{24} + C_2 \quad \Rightarrow \frac{wL^4 - 5wL^4}{120} + C_2 = 0$$

$$\Rightarrow C_2 = \left(-\frac{wL^4}{30} \right)$$

∴ for slope @ free end ($x=0$)

$$\Rightarrow EI(\theta_B) = C_1$$

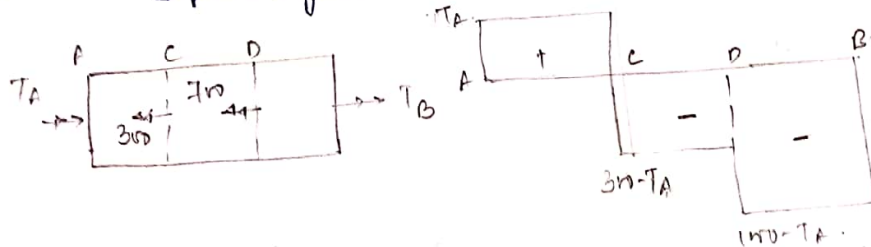
$$\Rightarrow \theta_B = \left(\frac{-Wl^3}{24EI} \right) (\downarrow)$$

@ $x=0$

$$\text{and } \delta_B = \frac{1}{EI} [C_2] = \left(\frac{-Wl^4}{30EI} \right) (\downarrow)$$



1-c) Torque diagram:



$$\sum F_x = 0$$

$$T_A + T_B = 1000 \text{ Nm} \quad \text{--- (1)}$$

from compatibility

$$\theta_{AD} = 0 = \left(\frac{T_A \times 2}{GJ} \right)_B + \left[\frac{-(300 - T_A) \times 2}{GJ} \right]_{A1} + \left[\frac{-(1000 - T_A) \times 2}{GJ} \right]_{2t} = 0$$

$$\frac{T_A \times 2 \times 10^6}{\frac{\pi}{32} \times 25^4 \times 35} + \frac{(T_A - 300) \times 2 \times 10^6}{28 \times \frac{\pi}{32} \times 50^4} + \frac{(T_A - 1000) \times 2 \times 10^6}{83 \times \frac{\pi}{32} \times 25^4} = 0$$

$$\Rightarrow [1.49T_A + 0.1164(T_A - 300) + 0.628(T_A - 1000)] = 0$$

$$2.2344T_A - 662.92 = 0$$

$$\Rightarrow T_A = \underline{296.88 \text{ Nm}}$$

$$\text{and } T_B = \underline{703.12 \text{ Nm}}$$

∴ Max shearing stresses:

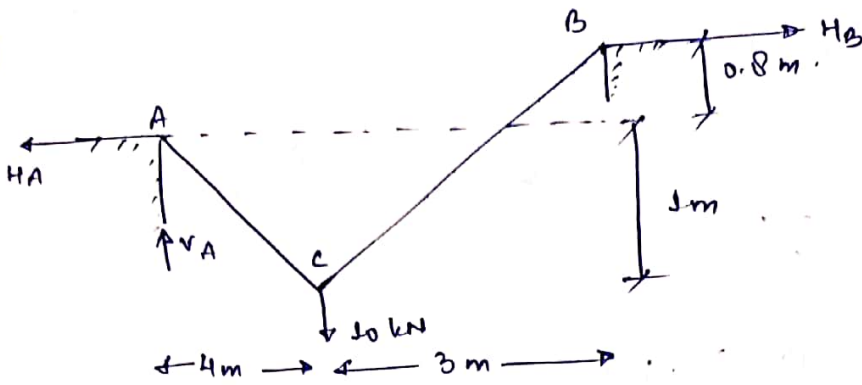
In Bronze: $\frac{16T_A}{\pi d^3} = \frac{16 \times 296.88 \times 10^3}{\pi \times 25^3} = \underline{96.768 \text{ MPa}}$

In Aluminium: $\frac{16 \times 703.12 \times 10^3}{\pi \times 50^3} = \underline{0.127 \text{ MPa}}$

In Steel: $\frac{16 \times 703.12}{\pi \times 25^3} = \underline{229.18 \text{ MPa}}$

(Ans)

1-d)



$$\sum M_B = 0$$

$$V_A \times 7 + H_A(1.8) = 10 \times 3 \Rightarrow 7V_A + 1.8H_A = 30 \quad \text{--- (1)}$$

also $\sum M_C = 0$ (L $\rightarrow$ R)

$$V_A \times 4 - H_A = 0 \Rightarrow \underline{4V_A = H_A}$$

$\therefore$ from eq (1)

$$V_A + 1.8(4V_A) = 30 \Rightarrow V_A = 2.941 \text{ kN}$$

$$\text{and } H_A = 4V_A = 11.765 \text{ kN}$$

and $\sum F_y = 0 \quad V_A + V_B = 10 \text{ kN}$

$$\Rightarrow V_B = 10 - V_A = 10 - 2.941 = \underline{7.059 \text{ kN}}$$

$$\sum M_C = 0 \quad (R \rightarrow L)$$

$$-V_B \times 3 + H_B(1.8) = 0$$

$$\Rightarrow H_B = \left(\frac{V_B \times 3}{1.8} \right) = \underline{11.765 \text{ kN}}$$

$\therefore$ Tension in segments

$$A_C = \sqrt{V_A^2 + H_A^2} = \sqrt{2.941^2 + 11.765^2} = \underline{12.127 \text{ kN}}$$

$$B_C = \sqrt{V_B^2 + H_B^2} = \sqrt{7.059^2 + 11.765^2} = \underline{13.72 \text{ kN}}$$

Support Reactions:

At A:

$$V_A = 2.941 \text{ kN}$$

$$H_A = 11.765 \text{ kN}$$

At B:

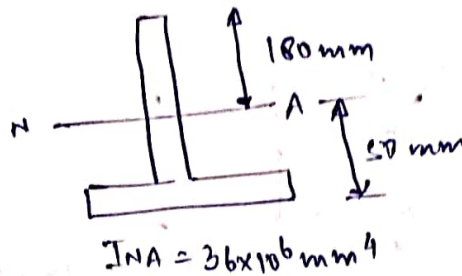
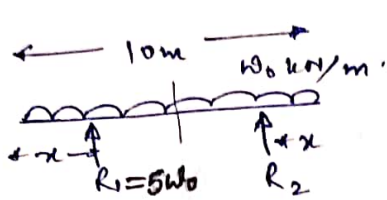
$$V_B = 7.059 \text{ kN}$$

$$H_B = 11.765 \text{ kN}$$

1-c) Processes involved in Manufacturing of Tiles:

- a) Preparation of clay: Preparation of clay involves cleaning and removing unwanted matter from the clay used for preparation.
- b) Blending / batching: Mixing ingredients in required proportions.
- c) Tempering: Process of inducing required plasticity to clay.
- d) Moulding: Process of giving required shape and size to moulds.
- e) drying: Process of removing the moisture either naturally or artificially to make it fit for next operation of burning.
- f) burning: Process of achieving hard and dense mass.

Q-4-a) i)

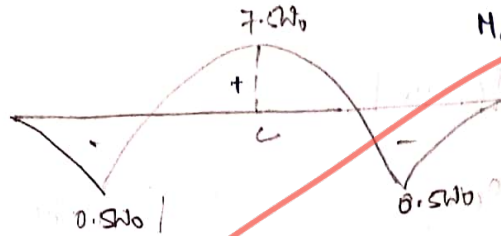


Given: $f_{bt} = 20 \text{ MPa}$
 $f_{bc} = 80 \text{ MPa}$

Soln: BMD

Sagging BM = $7.5w_0$

Hogging BM = $0.5w_0$



$$M_c(\text{center}) = 5w_0 \times 4 - w_0 \times 5 \times 2.5 = 7.5w_0 \text{ kNm}$$

$$\therefore \frac{7.5w_0 \times 10^6}{36 \times 10^6} \times 180 \leq f_{bc} (80)$$

for sagging BM

$$w_0 \leq 2.133 \text{ kN/m} \quad \text{--- (1)}$$

$$\frac{7.5w_0 \times 10^6}{36 \times 10^6} \times 50 \leq f_{bt} (20)$$

$$w_0 \leq 1.92 \text{ kN/m} \quad \text{--- (2)}$$

for hogging moment

$$\frac{0.5w_0 \times 10^6}{36 \times 10^6} \times 180 \leq f_{bt} (20)$$

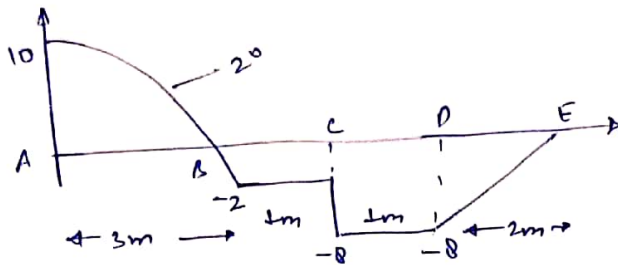
$$w_0 \leq 8 \text{ kN/m} \quad \text{--- (3)}$$

$$\frac{0.5w_0 \times 10^6}{36 \times 10^6} \times 50 \leq f_{bc} (80)$$

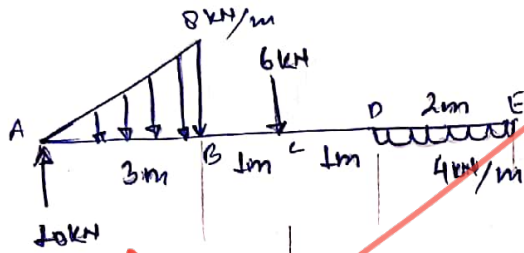
$$\Rightarrow w_0 \leq 115.2 \text{ kN/m} \quad \text{--- (4)}$$

$\therefore$ from above values $w_{\text{max}} = 1.92 \text{ kN/m}$ (Ans)

4-a-ii) SFD



2nd: Loading diagram



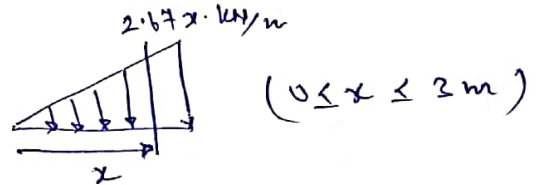
Bending Moments

$$M_A = 0 \text{ kNm}$$

$$\therefore M_x = 10 \times 2.737 - \frac{1}{2} \times 8 \times 2.737 \times \frac{2.737}{3} \downarrow$$

$$= 20.528 \text{ kNm}$$

Let @ x SF be zero from (A)



$$\therefore SF_x = 10 - \frac{1}{2} \times x \times 2.67x = 0$$

$$M_B = 10 \times 3 - \frac{1}{2} \times 8 \times 3 \times \frac{3}{2} \downarrow = 18 \text{ kNm}$$

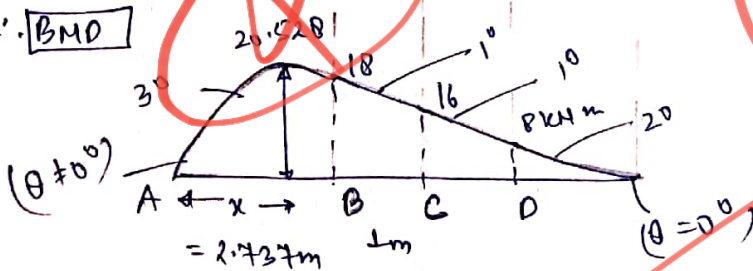
$$\Rightarrow 10 = \frac{1}{2} \times 2.67x^2$$

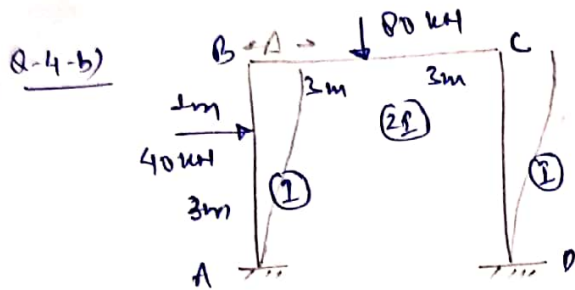
$$\Rightarrow x = 2.737 \text{ m} < 3 \text{ m (OK)}$$

$$M_C = 10 \times 4 - \frac{1}{2} \times 8 \times 3 \times \left(\frac{3}{3} + 1\right) = 16 \text{ kNm}$$

$$M_D = 10 \times 5 - \frac{1}{2} \times 8 \times 3 \times \left(\frac{3}{3} + 2\right) - 6 \times 1 = 8 \text{ kNm}$$

BMD





① $P_k = 3 (\theta_B, \theta_C, \Delta_x)$

② FEM's.

$$M_{FAB} = -\frac{wab^2}{12} = -\frac{40 \times 3 \times 1^2}{12} = -7.5 \text{ kNm}$$

$$M_{FBA} = 22.5 \text{ kNm}$$

$$M_{FBC} = -60, \quad M_{FCB} = 60$$

$$M_{FCD} = M_{FDC} = 0 \text{ kNm}$$

③ for θ_B , $\sum M_B = 0 \Rightarrow \underline{M_{BA} + M_{BC} = 0} \quad \text{--- (1)}$

$$M_{BA} = 22.5 + \frac{2EI}{4} (2\theta_B + 0 - \frac{3\Delta}{4}) = 22.5 + EI\theta_B - 0.375EI\Delta$$

$$M_{BC} = -60 + \frac{2EI(2)}{6} (2\theta_B + \theta_C) = -60 + 1.33EI\theta_B + 0.67EI\theta_C$$

$\therefore$ from eq (1)

$$\underline{2.33EI\theta_B + 0.67EI\theta_C - 0.375EI\Delta = 37.5} \quad \text{--- (A)}$$

④ for θ_C , $\sum M_C = 0 \Rightarrow \underline{M_{CB} + M_{CD} = 0} \quad \text{--- (2)}$

$$M_{CB} = 60 + \frac{2EI(2)}{6} (2\theta_C + \theta_B) = 60 + 1.33EI\theta_C + 0.67EI\theta_B$$

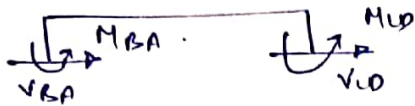
$$M_{CD} = 0 + \frac{2EI}{4} (2\theta_C + 0 - \frac{3\Delta}{4}) = EI\theta_C - 0.375EI\Delta$$

$\therefore$ from eq (2)

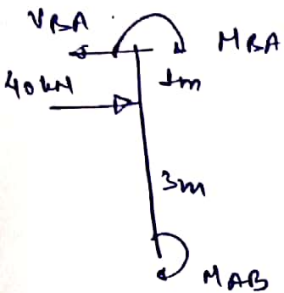
$$\underline{0.67EI\theta_B + 2.33EI\theta_C - 0.375EI\Delta = -60} \quad \text{--- (B)}$$

⑤ for Δx , $\sum F_x = 0$

$$\Rightarrow \overline{V_{BA} + V_{LD} = 0} \quad - (3)$$



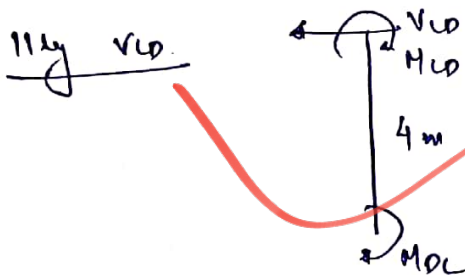
for V_{BA} :



$$\sum H_A = 0$$

$$-V_{BA} \times 4 + M_{AB} + M_{BA} + 40 \times 3 = 0$$

$$\Rightarrow V_{BA} = \left(\frac{M_{AB} + M_{BA} + 120}{4} \right)$$



$$\Rightarrow V_{LD} = \frac{M_{LD} + M_{DC}}{4}$$

$\therefore$ from eq (3)

$$M_{AB} + M_{BA} + M_{LD} + M_{DC} = -120 \quad - (C)$$

where $M_{AB} = -7.5 + \frac{2EI}{4} \left(\theta_B - \frac{3\Delta}{4} \right) = -7.5 + 0.5EI\theta_B - 0.375EI\Delta$

$$M_{DC} = 0 + \frac{2EI}{4} \left(0 + \theta_C - \frac{3\Delta}{4} \right) = 0.5EI\theta_C - 0.375EI\Delta$$

$\therefore$ from eq (C)

$$(-7.5 + 0.5EI\theta_B - 0.375EI\Delta) + (-7.5 + EI\theta_C - 0.375EI\Delta) + (EI\theta_C - 0.375EI\Delta + 0.5EI\theta_C - 0.375EI\Delta) = -120$$

$$15 + 1.5EI\theta_B - 1.5EI\Delta + 1.5EI\theta_C = -120$$

$$\Rightarrow \underline{1.5EI\theta_B + 1.5EI\theta_C - 1.5EI\Delta = -135} \quad - (D)$$

solving eq (A), (B) and (D) we get:

$$\theta_B = \frac{39.367}{EI}, \quad \theta_C = \frac{-19.367}{EI}, \quad \Delta = \frac{110}{EI}$$

⑥ FINAL END MOMENTS:

$$M_{AB} = -29.07 \text{ kNm}$$

$$M_{BA} = 20.617 \text{ kNm}$$

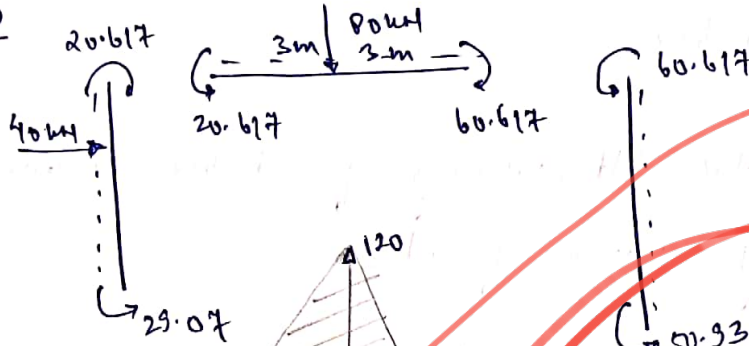
$$M_{BC} = -20.617 \text{ kNm}$$

$$M_{CB} = 60.617 \text{ kNm}$$

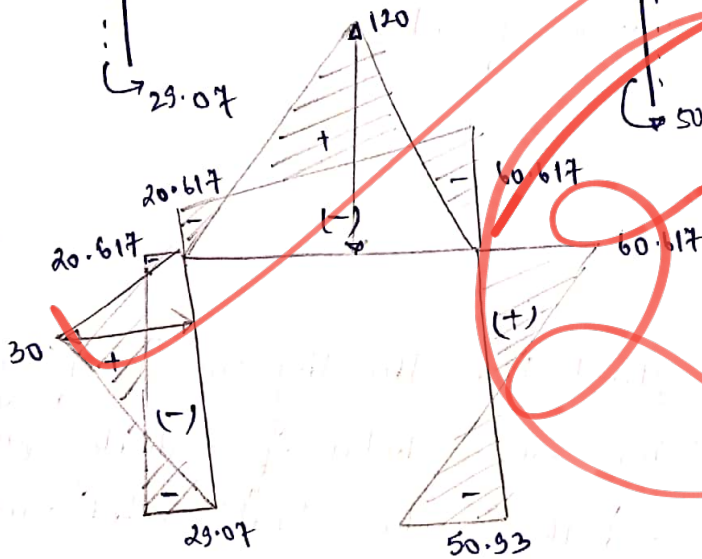
$$M_{CD} = -60.617 \text{ kNm}$$

$$M_{DC} = 50.93 \text{ kNm}$$

⑦ FBD



⑧ PMO

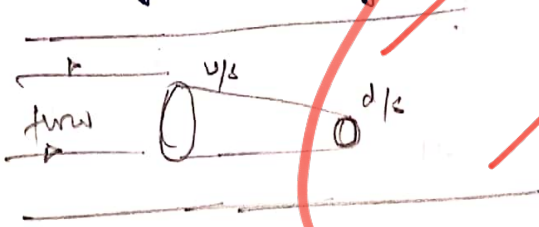


Q-4-ii) Methods of Artificial seasoning of timber are:

- Boling: It is one of the quickest method of seasoning of timber but sections obtained from this method is liable to wist and warp because of improper seasoning.
- Chemical/salt seasoning: In this method timber is soaked in a soln of chemical such as ZnCl_2 or HgCl_2 due to which heartwood which is dead and dry absorbs chemicals faster and hence a uniform shrinkage is attained at interior and outer parts of the timber.

c) Kiln seasoning: In this method of seasoning timber sections are placed in an air tight kiln in which saturated air is passed through. The timber sections placed at sufficient spacing between them. Saturated air at pressure of 21 kg/cm^2 is maintained for few hours and then the moisture inside the chamber is then reduced which causes drying of timbers.

d) Water seasoning: In this method seasoning of timber section is carried out by weathering action of water.



e) Electric seasoning: Timber is placed b/w alternating currents of high frequency and the timber being non conductor of electricity provides resistance to flow of current and thus produces heat, which causes seasoning of timber.

4-c-ii) Mechanical treatment of steel involves all those treatments which are given to steel by manual or machine methods to give it required shape and size, so that it can be directly used in engineering constructions.

Various Mechanical Treatments given to steel are:

1) Forging: In this process steel is beaten with the help of hammer and Anvil to remove the surface strains if any.

- 2) Drawing: The process of reducing the ϕ s of a steel specimen and turning it into wire is called drawing of steel.
- 3) Pressing: In this method steel is pressed in die and punch to given the molten ingots into specific shape and size.
- 4) Milling: The process of rolling the steel sections in a mechanical mill or rollers to give them the desired compact structure.

Section - B

5-a) Limit state is a state which implies that the structure is no longer able to perform its satisfied function.

As per IS: 800-2007, there are two types of limit states:

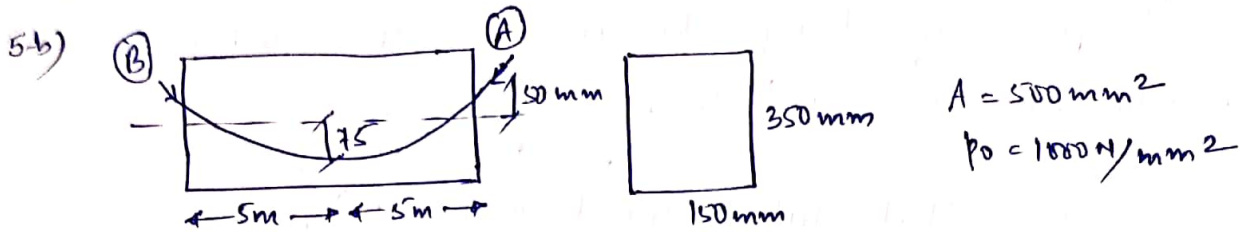
1) Limit state of strength: In this state, the structure is loaded to such a worse combination of loads that failure of structure occurs. The limit state of strength includes:

- Loss of stability
- Yielding
- Collapse

2) Limit state of serviceability: This limit state describes or deals with the performance of the given structure against permissible limits.

This limit state includes:

- deflection (excessive)
- vibration, racking or corrosion
- ponding of structure
- fatigue failure



Soln: effective eccentricity @ Mid span = $(50 + 50) = 100 \text{ mm}$.

$$\therefore p_k(\text{loss}) = p_0 (kx + 4x)$$

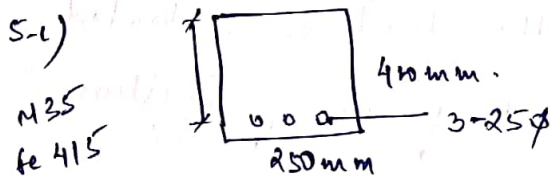
$$\text{@ } x = 10 \text{ m and } \alpha = \left(\frac{8h}{L} \right) = \frac{8 \times 0.125}{10} = 0.1$$

$$\therefore p_k = p_0 (0.0015 \times 10 + 0.1 \times 0.55) = 0.07 p_0$$

$$\therefore p_0 = p_0 - p_k = 0.93 p_0$$

$$\therefore p_0 (\text{other end}) = 0.93 \times 1000 \times 500 \times 10^{-3} = 465 \text{ kN}$$

$$\therefore \text{loss of prestress} = \frac{0.07 p_0}{p_0} \times 100 = 7\% \quad (\text{Ans})$$



Soln: ① $A_{st} = 3 \times \frac{\pi}{4} \times 25^2 = 1472.62 \text{ mm}^2$

② Assuming URS:

$$\therefore \text{from } [C\text{-force} = T\text{-force}]$$

$$\Rightarrow 0.36 \times 25 \times 250 \times x_u = 0.87 \times 415 \times 1472.62$$

$$x_u = 168.75 \text{ mm}$$

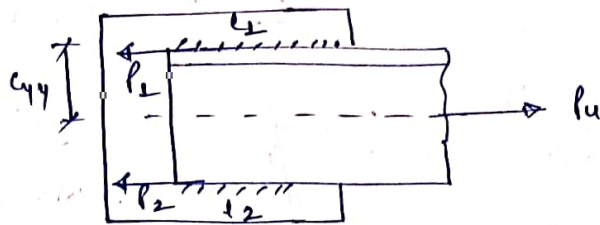
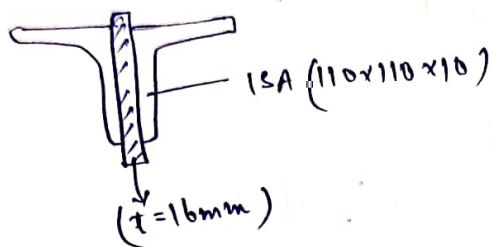
$$\text{where } x_{ulim} = kd = 0.48 \times d = 0.48 \times 400 = 192 \text{ mm}$$

$$\therefore x_u < x_{ulim} \quad (\text{URS, ok})$$

$$\therefore (MOR)_{beam} = 0.87 f_y A_{st} (d - 0.42 x_u) = [0.87 \times 415 \times 1472.62 \times (400 - 0.42 \times 168.75) \times 10^{-6}]$$

$$MOR = 174.983 \text{ kNm} \quad (\text{Ans})$$

5-d)



Ans:

→ let the weld lengths be l_1 and l_2 respectively

$$P_u = (\text{factored loads}) = 640 \times 1.5 = \underline{960 \text{ kN}}$$

∴ load carried through one ISA will be $\left(\frac{960}{2}\right) = \underline{480 \text{ kN}}$

from $\sum x = 0$

$$P_1 + P_2 = P_u = 480 \quad \text{--- (1)}$$

$$(\sum M)_C = 0 \Rightarrow -P_1(44) + P_2(110 - 44) = 0$$

$$\Rightarrow P_1(30.9) = P_2(110 - 30.9)$$

$$30.9 P_1 = 79.1 P_2$$

$$\Rightarrow \underline{P_1 = 2.56 P_2} \quad \text{--- (2)}$$

$$\Rightarrow \underline{P_1 = 345.168 \text{ kN}} \quad \text{and} \quad \underline{P_2 = 134.83 \text{ kN}}$$

Size of weld =

$$(\text{at } \textcircled{1}) = 10 - 1.5 = 8.5 \text{ mm}$$

@ location (2)

$$s = \frac{3}{4}t = \frac{3}{4} \times 10 = 7.5$$

∴ Assume $\underline{s = 5 \text{ mm}}$ all around.

∴ length of weld at $\textcircled{1}$ $(K_s)_{\text{eff}} \times \frac{40}{\sqrt{3} \times 1.25} \geq P_1 (345.168)$

$$(0.7 \times 5) \times l_{\text{eff}1} \times \frac{40}{\sqrt{3} \times 1.25} \geq 345.168 \times 10^3$$

$$(l_{\text{eff}1}) \geq 533.73 \text{ A } \underline{535 \text{ mm (say)}}$$

length of weld @ $\textcircled{2}$

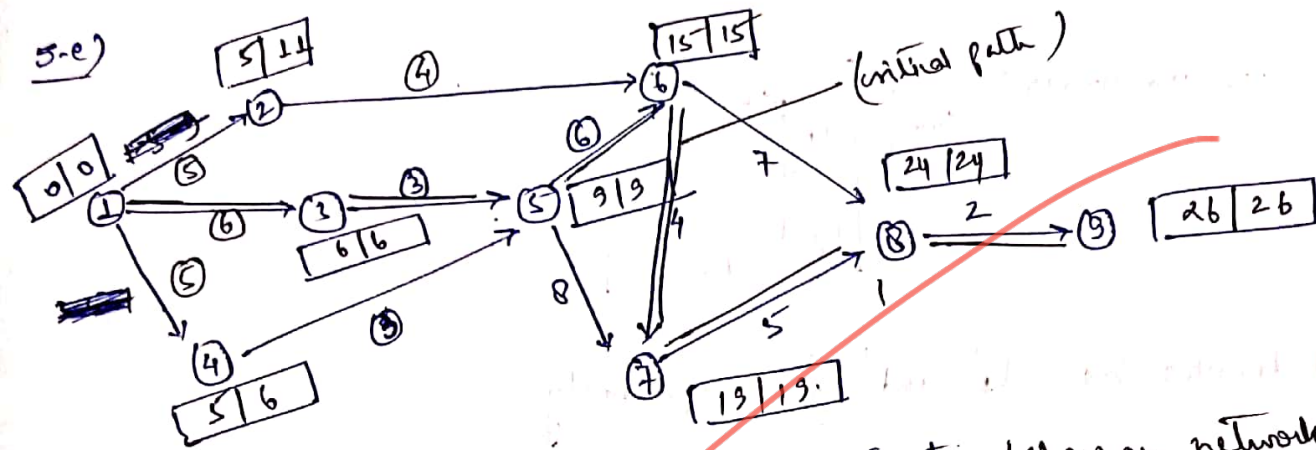
$$0.7 \times 5 \times l_{\text{eff}2} \times \frac{40}{\sqrt{3} \times 1.25} \geq 134.83 \times 10^3$$

$$(l_{\text{eff}2}) \geq 208.51 \text{ mm}$$

$$\Rightarrow \underline{(l_{\text{eff}2})_2 = 210 \text{ mm (say)}}$$

Ans.

5-e)



Activity

$$TE = (To + 4 \times \text{duration} + TF) / 6$$

Event (show on network itself)

1-2

5

1-4

5

1-3

6

2-6

4

3-5

3

5-6

6

5-7

8

6-7

4

6-8

7

7-8

5

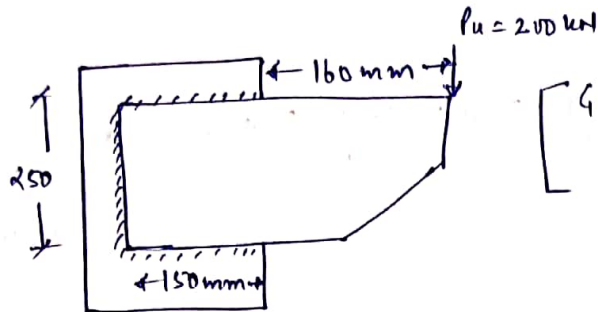
8-9

2

4-5

3

Q-6-a)

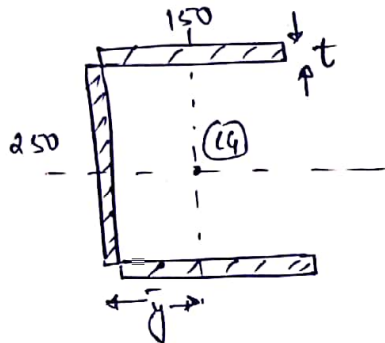


Given $(f_u) = 410 \text{ MPa}$
Shop Weld

Soln:

Weld qe.

(Treating I_2 , t^2 and I^2 as negligible in analysis)



$$\bar{y} = \frac{(150 \times t \times 75 \times 2) + (250 \times t \times \frac{t}{2})}{(250 \times t + 150 \times t \times 2)} = 40.91 \text{ mm}$$

$$\therefore \text{Torsion @ Weld} = 200 \times (160 + (150 - 40.91)) = 53818.18 \text{ kNm}$$

$$\text{and eccentricity (e)} = (160 + 150 - 40.909) = 269.09 \text{ mm}$$

$$(I_{xx})_{\text{Weld}} = 2 \times \left[\frac{150 \times t^3}{12} + 150 \times t \times 125^2 \right] + t \times \frac{250^3}{12} = 5.989 \times 10^6 t \text{ mm}^4$$

$$(I_{yy})_{\text{Weld}} = 2 \times \left[t \times \frac{150^3}{12} + 150 \times t \times (75 - 40.91)^2 \right] + \left[t \times \frac{250^3}{12} + 250 \times t \times (40.91^2) \right]$$

$$= 1.329 \times 10^6 t \text{ mm}^3$$

$$(I_p)_{\text{Weld}} = (I_{xx} + I_{yy}) = 7.318 \times 10^6 t \text{ mm}^4$$

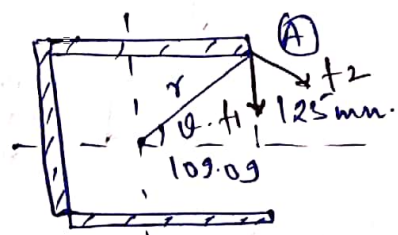
→ Force / stress Analysis (At point A)

$$f_1 (\text{direct shear stress}) = \left(\frac{200 \times 10^3}{150 \times t \times 2 + 250 \times t} \right) = \frac{363.64}{t} \text{ N/mm}^2$$

$$f_2 (\text{torsional shear}) = \left(\frac{T}{I_p} \times r \right)$$

$$\text{where } r = \sqrt{125^2 + 109.09^2} = 165.908 \text{ mm}$$

$$\therefore f_2 = \frac{53818.18 \times 10^3}{7.318 \times 10^6 t} \times 165.908 = \frac{1220.12}{t} \text{ N/mm}^2$$



$$\tan \theta = \frac{125}{109.09}$$

$$\Rightarrow \theta = 48.89^\circ$$

(10)

$$f_c = \sqrt{f_1^2 + f_2^2 + 2f_1 f_2 \cos \theta} = \sqrt{\left(\frac{363.64}{t}\right)^2 + \left(\frac{1220.12}{t}\right)^2 + 2 \times 363.64 \times 1220.12 \cos(48.83)} \quad \text{Ans (48.83)}$$

$$\Rightarrow f_c = \frac{1484.714}{t} \text{ N/mm}^2$$

and for weld to be safe in shear:

$$f_c = \frac{1484.714}{t} \leq \frac{f_u}{\sqrt{3} \times 1.25}$$

$$\Rightarrow t \geq 8.036 \text{ mm}$$

$$\therefore s \geq \left(\frac{8.036}{0.7}\right) = 11.48$$

$$\Rightarrow \underline{s = 12 \text{ mm}} \quad (\text{Ans.})$$

6-b) ISA $200 \times 150 \times 15$ length = 5m.

$$A_g = 5025 \text{ mm}^2$$

$$I_{xx} = 2 \times 10^4 \text{ mm}^4 \quad I_{yy} = 9.7 \times 10^6 \text{ mm}^4$$

$$\text{S.A.}: I_{xy} = 8.3 \times 10^6 \text{ mm}^4$$

$$I_{uu}/I_{vv} = \frac{I_{xx} + I_{yy}}{2} \pm \frac{1}{2} \sqrt{(I_{xx} - I_{yy})^2 + 4I_{xy}^2}$$

$$I_{uu}/I_{vv} = \frac{2 \times 10^4 + 9.7 \times 10^6}{2} \pm \frac{1}{2} \sqrt{(2 \times 10^4 - 9.7 \times 10^6)^2 + 4 \times (8.3 \times 10^6)^2}$$

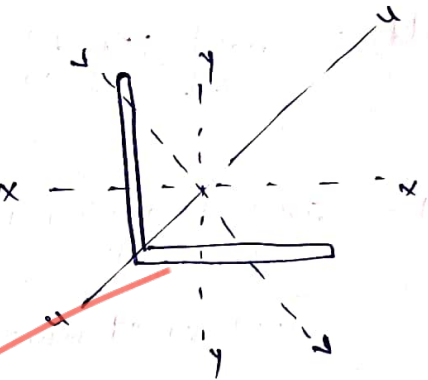
$$\Rightarrow \underline{I_{min} = I_{vv} = 5.082 \times 10^6 \text{ mm}^4}$$

$$\therefore I_{min} = A_g \times r_{min}^2$$

$$\therefore r_{min} = \sqrt{\frac{I_{min}}{A_g}} = \sqrt{\frac{5.082 \times 10^6}{5025}} = \underline{31.8018 \text{ mm}}$$

$$\therefore \lambda = \left(\frac{kl}{r_{min}}\right) \quad \left(\text{Assuming discontinuous strut}\right)$$

$$\therefore \lambda = \left(\frac{0.85 \times 5 \times 10^3}{31.8018}\right) = \underline{133.64}$$



$$\therefore f_d = 57 - \frac{57-51}{140-130} (133.64 - 130) = 54.815 \text{ N/mm}^2$$

$$\therefore \text{factored load carrying capacity} = (f_d \times A_g) = (54.815 \times 5025 \times 10^{-3}) = 275.449 \text{ kN}$$

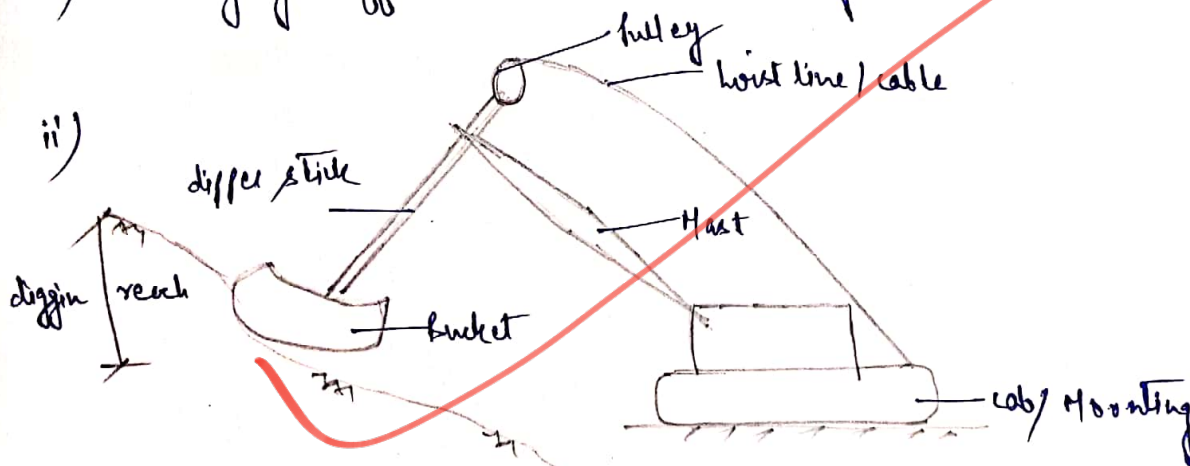
$$\therefore \text{safe} = \left(\frac{P_c}{1.5} \right) = \left(\frac{275.449}{1.5} \right) = 183.633 \text{ kN} \quad (\text{Ans})$$

Q-6-c) i) Dragline:

- Purpose:
- It is used for excavation of soft soils and fills.
 - It has cutting edge that is used to loosen the soil with impact and dragging the bucket against the earth to collect it.
 - It can be used for excavation of earth both above and below the track level.

Advantages of Dragline:

- It can be used for excavation in soils below water table.
- Digging reach of dragline is comparatively more than that of power shovels.
- Draglines can directly be employed into the trenches.
- Highly efficient under close range works.



Typical sketch of dragline.

iii) Factors affecting output of a dragline:

a) Size of Bucket: Larger the size of the bucket more will be the material handled and thus lesser will be the cycle time.

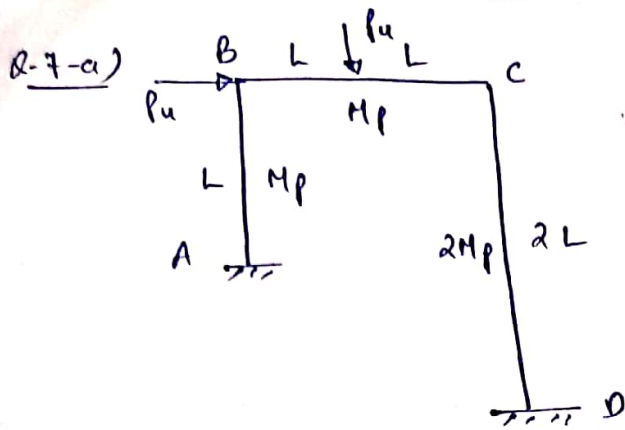
b) Material to be dug: Harder materials offers greater resistance to excavation and vice-versa.

c) Depth of cut and angle of swing: Higher will be the depth of cut more will be the material excavated in single operation.

$$\eta (\text{efficiency}) \propto \left(\frac{1}{\text{Angle of swing}} \right)$$

d) Size of hauling units: Larger the size of hauling units, lesser will be the delay in emptying the earth and greater will be the output.

e) Skill of operator: The output of various excavating equipments depends on the skill and the proficiency of the operator handling it.



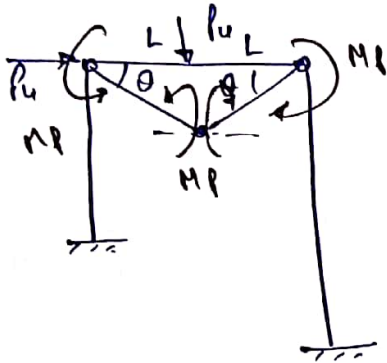
$$\textcircled{1} \quad D_s = 6 - 3 = 3$$

$$\Rightarrow (D_s + 1) = n = 4 \text{ plastic hinges}$$

$$\textcircled{2} \quad K = N - D_s = 5 - 3 = \textcircled{2}$$

(In addition to above 2 mechanisms, there will also be a combined mechanism)

① BEAM MECHANISM:



from principle of virtual work

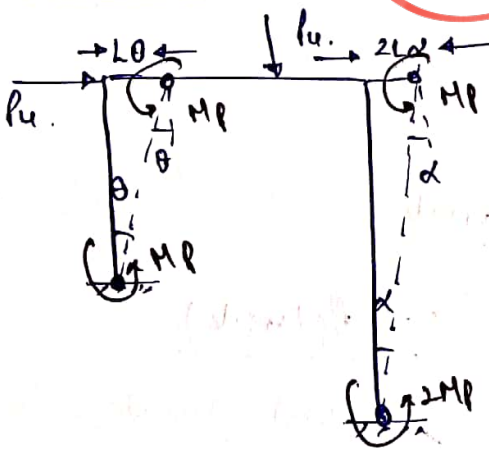
$$\text{Internal WD} = \text{External WD}$$

$$M_p\theta + M_p(\theta + \theta) + M_p\theta = P_u \times L\theta$$

$$\Rightarrow 4M_p\theta = P_u \times L\theta$$

$$\Rightarrow P_u = \left(\frac{4M_p}{L} \right) \quad \text{--- (1)}$$

② SWAY MECH:



from strain comp:

$$L\theta = 2L\alpha$$

$$\Rightarrow \boxed{\theta = 2\alpha}$$

$\therefore$ from virtual work principle:

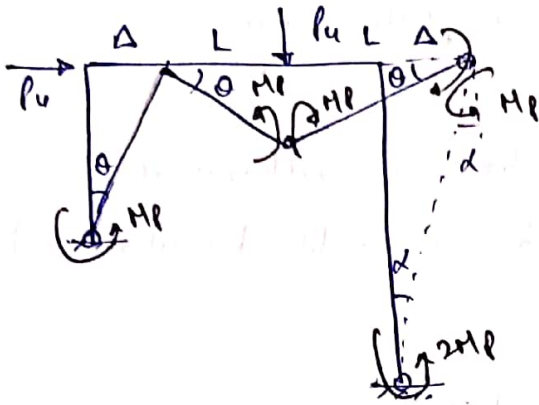
$$M_p\theta + M_p\theta + M_p\alpha + 2M_p\alpha = P_u \times L\theta$$

$$2M_p\theta + M_p\left(\frac{\theta}{2}\right) + M_p\theta = P_u \times L\theta$$

$$3.5M_p\theta = P_u \times L\theta$$

$$\Rightarrow P_u = \left(\frac{3.5M_p}{L} \right) \quad \text{--- (2)}$$

③ Combined Mechanism:



$$L\theta = 2L\alpha = \Delta$$

$$\Rightarrow \boxed{\alpha = 0.5\theta}$$

$\therefore$ from virtual work mtd:

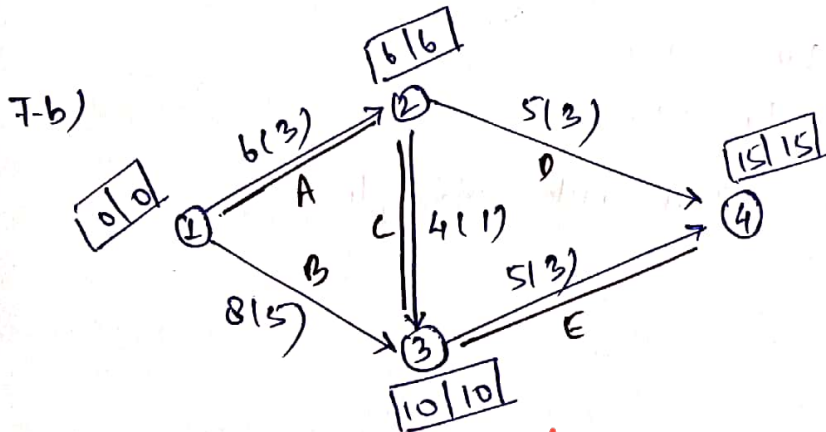
$$P_u \times L\theta + P_u \times 2L\theta = M_p\theta + M_p(\theta + \theta) + M_p\theta + M_p\alpha + 2M_p\alpha$$

$$2P_u \times 2L\theta = 4M_p\theta + 3M_p(0.5\theta)$$

$$\therefore P_u = \left(\frac{5.5M_p}{2L} \right) = \left(\frac{2.75M_p}{L} \right) \quad \text{--- (3)}$$

$\therefore$ from above mechanisms $P_u = \min(1, 2, 3) = \left(\frac{2.75M_p}{L} \right)$ (Ans)

and correct failure mechanism is combined Mechm.



$$IC = 3000/\text{week.}$$

Activity	CS (Rs/week)
1-2 (A)	2500
1-3 (B)	1500
2-3 (C)	1000
2-4 (D)	3500
3-4 (E)	3000

Soln:

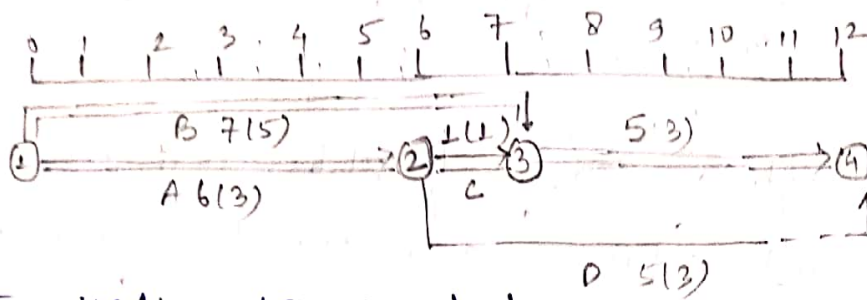
Basic Step ($T_n = 15$ days)

$$DC = Rs\ 30,000$$

$$IC = 3000 \times 15 = Rs\ 45,000$$

$$\underline{TC = Rs\ 75,000}$$

TIME SCALE DIAGRAM :



Ist stage washing ($T_n = 13$ weeks)

wash (activity C by 2 weeks)

$\therefore T_n = 13$ weeks

$$P_c = 30,000 + 2 \times 1,000 = 32,000$$

$$I_c = 13 \times 3,000 = 39,000$$

$$TC = \text{Rs } 71,000$$

~~IInd stage washing~~ ($T_n = 12$ weeks)

~~wash activity (B and C by 1 week each)~~

$$P_c = 32,000 + 1,500 + 1,000 = 34,500$$

$$I_c = 12 \times 3,000 = 36,000$$

$$TC = \text{Rs } 70,500$$

IIIrd stage washing ($T_n = 11$ weeks)

wash (activity E by 2 weeks)

$$\therefore P_c = 32,000 + 3,000 \times 2 = 38,000$$

$$I_c = 3,000 \times 11 = 33,000$$

$$TC = \text{Rs } 71,000$$

IVth

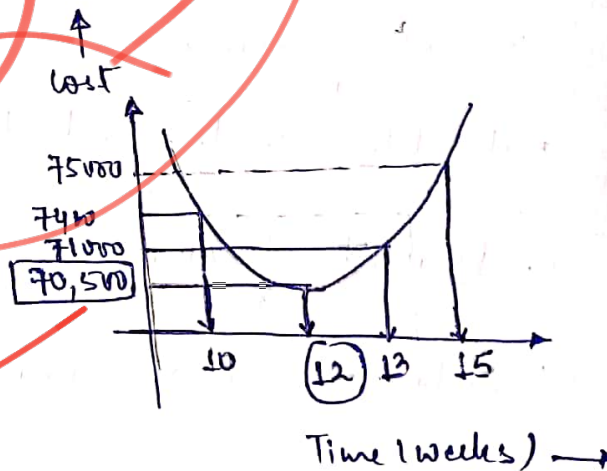
IVth stage washing ($T_n = 10$ weeks)

wash activity (B, C, and D by 1 week each)

$$\therefore P_c = 38,000 + 1,500 + 1,000 + 3,500 = 44,000$$

$$I_c = 3,000 \times 10 = 30,000 = 30,000$$

$$\therefore TC = \text{Rs } 74,000$$



$\therefore$ optimum project duration = 12 days
Minimum cost = Rs 70,500] (Ans)

Q-7-c) Flat slab: → Beams of great depth reduce due to construction of flat slab.

→ Flat slab are the thin ordinary reinforced slabs that are ~~mono~~ monolithically casted with columns and doesn't require any beam to support from base.

→ Column heads enlarged at the location where ~~sub~~ slab is present called as column heads or capitals and the slab is made usually thick at the interface called as drop panels.

→ Load transfer in flat slab directly takes place from slab to column.

Advantages of Flat slab:

- 1) Construction of flat slab reduces clear height b/w the floor levels and thus reducing the ~~story~~ story height.
- 2) It resists fire better than ordinary slab.
- 3) Construction is faster as it eliminates the removal of formwork in case of beams.
- 4) Flat slabs resist point load better.
- 5) Dead weight of structure reduces greatly.
- 6) Concrete is more economically used in construction of flat slabs.

