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CLASS TEST 2019-2020

MECHANICAL ENGINEERING

Date of Test : 17/08/2019**ANSWER KEY ➤ Strength of Material**

- | | | | | |
|--------|---------|---------|---------|---------|
| 1. (c) | 7. (c) | 13. (d) | 19. (b) | 25. (b) |
| 2. (c) | 8. (a) | 14. (d) | 20. (a) | 26. (c) |
| 3. (b) | 9. (a) | 15. (a) | 21. (c) | 27. (b) |
| 4. (c) | 10. (d) | 16. (a) | 22. (d) | 28. (d) |
| 5. (b) | 11. (d) | 17. (c) | 23. (a) | 29. (c) |
| 6. (c) | 12. (b) | 18. (a) | 24. (c) | 30. (b) |

DETAILED EXPLANATIONS

1. (c)

Thermal stress is produced only when it is restricted to expand and there exists temperature gradient.

3. (b)

$$\tau = \frac{16T}{\pi d^3} \quad \{T \rightarrow \text{torsional moment}, d \rightarrow \text{diameter}\}$$

For same T , we have

$$\begin{aligned} \tau_1 d_1^3 &= \tau_2 d_2^3 \\ \tau_2 &= \tau_1 \left(\frac{d_1}{d_2} \right)^3 = 128 \left(\frac{1}{2} \right)^3 = \frac{128}{8} = 16 \text{ MPa} \end{aligned}$$

4. (c)

$$\tau_{\max} = \frac{Pd}{4t} = \frac{4 \times 1000}{4 \times 10} = 100 \text{ MPa}$$

5. (b)

In case of unsymmetrical bending, the equation of the neutral axis is found out by finding the locus of the points on which the resultant stress is zero.

6. (c)

$$\sigma_1 + \sigma_2 = \sigma_x + \sigma_y = 40 + 60 = 100 \text{ MPa}$$

9. (a)

$$\text{Volumetric strain} = \frac{\sigma}{E} (1 - 2\nu) = \epsilon (1 - 2\nu) = 0.02 (1 - 2 \times 0.3) = 0.008$$

11. (d)

Let us use suffix 1 for uniform bar and suffix 2 for non-uniform bar.

$$\begin{aligned} U_1 &= \frac{P^2 L}{2A_1 E_1} = \frac{P^2 L}{2 \left(\frac{\pi}{4} D^2 \right) E_1} = \frac{2P^2 L}{\pi D^2 E_1} \\ U_2 &= \frac{P^2}{2E_2} \left[\frac{\frac{3}{4}L}{\frac{\pi}{4}D^2} + \frac{\frac{1}{4}L}{\frac{\pi}{4} \left(\frac{D}{2} \right)^2} \right] = \frac{P^2}{2E_2} \left[\frac{3L}{\pi D^2} + \frac{4L}{\pi D^2} \right] = \frac{7P^2 L}{2\pi D^2 E_2} \\ \therefore \frac{U_1}{U_2} &= \frac{4E_2}{7E_1} = \frac{4}{7} \times \frac{7}{4} = 1 \end{aligned}$$

14. (d)

Strain energy for Case-I:

$$U_I = \frac{\sigma^2}{2E} AL$$

Strain energy for Case-II:

$$\begin{aligned} U_{II} &= \left(\frac{\sigma}{4} \right)^2 \times \frac{1}{2E} \times 4A \times \frac{L}{4} \times 2 + \frac{\sigma^2}{2E} A \left(\frac{L}{2} \right) \\ &= \frac{\sigma^2}{2E} AL \left[\frac{1}{16} \times 4 \times \frac{1}{4} \times 2 \right] + \frac{\sigma^2}{2E} A \left(\frac{L}{2} \right) = \frac{\sigma^2}{2E} AL \left[\frac{1}{8} + \frac{1}{2} \right] = \frac{\sigma^2}{2E} A \left(\frac{5}{8} \right) = \left(\frac{5}{8} \right) U_I \end{aligned}$$

Strain energy for Case-III:

$$U_{III} = \frac{\sigma^2}{2E} \times A \times \frac{L}{4} \times 2 + \left(\frac{\sigma}{4}\right)^2 \times \frac{1}{2E} \times 4A \times \frac{L}{2}$$

$$= \frac{\sigma^2}{2E} AL \left[\frac{1}{2} + \frac{1}{16} \times 2 \right] = \frac{\sigma^2}{2E} AL \left[\frac{1}{2} + \frac{1}{8} \right] = \left(\frac{5}{8} \right) U$$

15. (a)

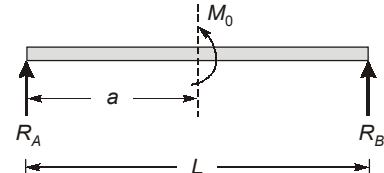
$$\sum M_A = 0, R_B \times L + M_0 = 0$$

$$R_B = -\frac{M_0}{L}$$

$$\sum F_y = 0, R_A + R_B = 0$$

So,

$$R_A = \frac{M_0}{L}$$



So option I is correct representation of shear force diagram.

16. (a)

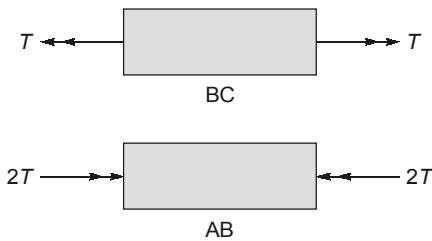
$$\theta_{BC} = \frac{T\left(\frac{L}{2}\right)}{GJ} = \frac{1}{2} \frac{TL}{GJ}$$

$$\theta_{AB} = \frac{(2T)\left(\frac{L}{2}\right)}{GJ} = \frac{TL}{GJ}$$

$$\theta_C = \theta_{BC} + \theta_{AB} = \frac{TL}{GJ}(1+0.5) = 1.5 \frac{TL}{GJ}$$

$$\theta_B = \frac{TL}{GJ} \text{ (As } \theta_A = 0)$$

$$\theta_B : \theta_C = 1 : 1.5$$



17. (c)

$$f = 12 \text{ Hz}, P = 20 \text{ kW} = 20000 \text{ N.m/s}$$

Torque,

$$T = \frac{P}{2\pi f} = \frac{20000}{2\pi \times 12} = 265.3 \text{ N.m}$$

$$\tau_{max} = \frac{16T}{\pi d^3} = \frac{16 \times 265.3 \times 10^3}{\pi \times (30)^3} \approx 50 \text{ MPa}$$

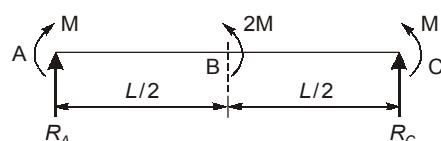
18. (a)

$$R_A + R_C = 0$$

$$R_A = -R_C$$

$$R_A = \frac{2M}{L} (\uparrow)$$

$$R_C = \frac{2M}{L} (\downarrow)$$



Shear force variation between AB and BC is constant.

$$M_A = M \text{ (clockwise)}$$

$$M_C = M \text{ (anti-clockwise)}$$

So option (a) is correct answer.



19. (b)

$$E = 200 \text{ GPa}, d = 1.25 \text{ mm}, R_0 = 500 \text{ mm}$$

$$\sigma = \frac{E \frac{d}{2}}{\rho} \quad (\rho - \text{radius of curvature})$$

$$\sigma_{\max} = \frac{\frac{E\frac{d}{2}}{R_0 + \frac{d}{2}}}{2} \quad \left(\rho_{\max} = R_0 + \frac{d}{2} \right)$$

$$= \frac{200 \times 10^3 \times 1.25}{2 \times 500 + 1.25} = 249.688 \approx 250 \text{ MPa}$$

20. (a)

UDL,

$$q = 5.8 \text{ kN/m}, b = 140 \text{ mm}, h = 240 \text{ mm}, L = 4 \text{ m}$$

$$\text{Maximum bending moment, } M_{\max} = \frac{qL^2}{8} = \frac{5.8 \times 10^3 \times 16}{8} = 11.6 \times 10^3 \text{ N.m}$$

Section modulus,

$$Z = \frac{I}{\frac{h}{2}} = \frac{bh^3}{12} \times \frac{2}{h} = \frac{bh^2}{6}$$

$$\sigma_{\max} = \frac{M_{\max}}{Z} = \frac{6M_{\max}}{bh^2} = \frac{6 \times 11.6 \times 10^6}{140 \times (240)^2} = 8.63 \text{ MPa}$$

21. (c)

$$M_{\max} = P(6b - 2b) = 4Pb$$

$$\sigma_a = \frac{M_{\max} \left(\frac{d_{\min}}{2} \right)}{\frac{\pi d_{\min}^4}{64}} \Rightarrow d_{\min} = \left(\frac{128Pb}{\pi \sigma_a} \right)^{1/3}$$

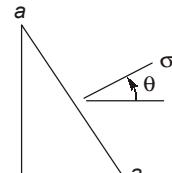
$$d_{\min} = \left(\frac{128 \times 40 \times 37}{\pi \times 30} \right)^{1/3} = 12.62 \text{ mm}$$

22. (d)

$$\sigma_x = 32 \text{ MPa}$$

$$\sigma_y = -50 \text{ MPa}$$

$$\tau_{xy} = 0$$

Normal stress on plane $a-a$ at an angle θ 

$$\sigma_{x1} = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$= \frac{32 - 50}{2} + \frac{32 + 50}{2} \cos 2\theta + 0 = -9 + 41 \cos 2\theta$$

$$\sigma_{x1} = 0 = -9 + 41 \cos 2\theta$$

$$\cos 2\theta = \frac{9}{41}$$

$$2\theta = 77.32^\circ \text{ or } \theta = 38.66^\circ$$

23. (a)

$$\tau_{\max} = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2} \right)^2 + \tau_{xy}^2}$$

$$\sigma_{y1} = \sigma_x + 2\sqrt{\tau_{\max}^2 - \tau_{xy}^2}$$

$$\sigma_{y2} = \sigma_x - 2\sqrt{\tau_{\max}^2 - \tau_{xy}^2}$$

$$\sigma_{y1} = 42 + 2\sqrt{(35)^2 - (33)^2} = 65.3 \text{ MPa}$$

$$\sigma_{y2} = 42 - 2\sqrt{(35)^2 - (33)^2} = 18.7 \text{ MPa}$$

So,

$$\sigma_y = \begin{pmatrix} 65.3 \\ 18.7 \end{pmatrix} \text{ MPa}$$

24. (c)

From stress-strain relations:

$$\epsilon_x = \frac{\sigma_x}{E} - \frac{\nu}{E}(\sigma_y + \sigma_z)$$

$$\epsilon_y = \frac{\sigma_y}{E} - \frac{\nu}{E}(\sigma_x + \sigma_z)$$

$$(-740 \times 10^{-6})E = -4.5 + 5.7\nu \quad \dots(i)$$

$$(-320 \times 10^{-6})E = -3.6 + 6.6\nu \quad \dots(ii)$$

Solving equation (i) and (ii), we get

$$E = 3000 \text{ MPa} = 3 \text{ GPa}, \nu = 0.4$$

Bulk modulus, $K = \frac{E}{3(1-2\nu)} = \frac{3}{3(1-2 \times 0.4)} = 5 \text{ GPa}$

25. (b)

$$\epsilon_A = 520 \times 10^{-6}, \epsilon_B = 360 \times 10^{-6}, \epsilon_C = -80 \times 10^{-6}$$

$$\epsilon_{x1} = \frac{\epsilon_x + \epsilon_y}{2} + \frac{\epsilon_x - \epsilon_y}{2} \cos 2\theta + \frac{\gamma_{xy}}{2} \sin 2\theta$$

For A, $\theta = 0^\circ, \epsilon_A = \epsilon_x = 520 \times 10^{-6} \quad \dots(i)$

For B, $\theta = 45^\circ, \epsilon_B = \frac{\epsilon_x + \epsilon_y}{2} + \frac{\epsilon_x - \epsilon_y}{2} \cos 90^\circ + \frac{\gamma_{xy}}{2} \sin 90^\circ$

$$\epsilon_B = \frac{\epsilon_x + \epsilon_y + \gamma_{xy}}{2} = 360 \times 10^{-6} \quad \dots(ii)$$

For C, $\theta = 90^\circ, \epsilon_C = \frac{\epsilon_x + \epsilon_y}{2} + \frac{\epsilon_x - \epsilon_y}{2} \cos 180^\circ + \frac{\gamma_{xy}}{2} \sin 180^\circ$

$$\epsilon_C = \epsilon_y = -80 \times 10^{-6} \quad \dots(iii)$$

From equation (i), (ii) and (iii), we get

$$\gamma_{xy} = 2\epsilon_B - \epsilon_A - \epsilon_C$$

$$= (2 \times 360 - 520 + 80) \times 10^{-6}$$

$$\gamma_{xy} = 280 \times 10^{-6}$$

Maximum shear strain, $\frac{\gamma_{xy}}{2} = \sqrt{\left(\frac{\epsilon_x - \epsilon_y}{2}\right)^2 + \left(\frac{\gamma_{xy}}{2}\right)^2}$

$$\frac{\gamma_{xy} \times 10^6}{2} = \sqrt{\left(\frac{520 + 80}{2}\right)^2 + \left(\frac{280}{2}\right)^2}$$

$$\gamma_{xy} = 2 \times 10^{-6} \sqrt{(300)^2 + (140)^2}$$

$$\gamma_{xy} = 662 \times 10^{-6}$$

26. (c)

Deflection due to point load, $\delta_P = \frac{PL^3}{3EI}$ (downward)

Deflection due to end moment, $\delta_M = \frac{ML^2}{2EI}$ (upward)

$$\begin{aligned}\delta &= \delta_P + \delta_M = -\frac{PL^3}{3EI} + \frac{ML^2}{2EI} \\ &= -\frac{100 \times 10^3 \times (9000)^3}{3 \times 81 \times 10^{12}} + \frac{900 \times 10^6 \times (9000)^2}{2 \times 81 \times 10^{12}} = -300 + 450 = 150 \text{ mm}\end{aligned}$$

27. (b)

Engineering stress, $\sigma = \frac{4P}{\pi d_0^2} = \frac{4 \times 50 \times 10^3}{\pi \times 100} = 636.62 \text{ MPa}$

Engineering strain, $\epsilon = \frac{\Delta l}{l_0} = \frac{1}{100} = 10^{-2}$

Engineering modulus, $E = \frac{\sigma}{\epsilon} = 636.62 \times 10^2 \text{ MPa} = 63.662 \text{ GPa}$
 $E = 2G(1 + \nu)$

$$\nu = \frac{E}{2G} - 1 = \frac{63.662}{2 \times 25} - 1 = 0.273$$

28. (d)

$$GC = CH = \frac{50 - 20}{2} = 15$$

$$\begin{aligned}OC &= OG + GC \\ &= 20 + 15 = 35\end{aligned}$$

$$FC = \sqrt{15^2 + 10^2} = 18.03$$

Coordinates of A $\equiv (35, 18.03) \text{ MPa}$

29. (c)

When the temperature drops, wire tends to contract due to fall in temperature. A wire is constrained at the end A and B and wire will be subjected to tensile stress.

$$\Delta T = 20 - 0 = 20^\circ\text{C}$$

$$\sigma = \sigma_1 + \sigma_2$$

$[\sigma_1 = 42 \text{ MPa} \text{ due to prestress}, \sigma_2 = E\alpha\Delta T \text{ due to temperature change}]$

$$= 42 + E\alpha\Delta T = 42 + 200 \times 10^3 \times 14 \times 10^{-6} \times 20 = 42 + 56 = 98 \text{ MPa}$$

30. (b)

$$\sigma_a = \frac{P}{A} = \frac{50 \times 10^3}{\frac{\pi}{4} \times 50^2} = 25.46 \text{ MPa}$$

$$\sigma_b = \frac{My}{I_{NA}} = \frac{50 \times 10^3 \times 10 \times 25 \times 64}{\pi \times (50)^4} = 40.74 \text{ MPa}$$

$$\tau_s = \frac{16T}{\pi d^3} = \frac{16 \times 500 \times 10^3}{\pi \times (50)^3} = 20.37 \text{ MPa}$$

According to MDET,

$$\sqrt{\sigma_x^2 + 3\tau_{xy}^2} = \frac{S_{yt}}{N}$$

$$N = 2.67$$

