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HIGHWAY ENGINEERING

CIVIL ENGINEERING

Date of Test : 26/09/2022

ANSWER KEY >

1. (b)	7. (c)	13. (a)	19. (d)	25. (b)
2. (d)	8. (d)	14. (a)	20. (a)	26. (b)
3. (a)	9. (c)	15. (d)	21. (d)	27. (c)
4. (b)	10. (c)	16. (a)	22. (b)	28. (c)
5. (c)	11. (d)	17. (b)	23. (c)	29. (a)
6. (a)	12. (d)	18. (c)	24. (a)	30. (b)

DETAILED EXPLANATIONS

1. (b)

Assuming V volume passed in peak hour. And for minimum peak hour volume, all volume of that hour, was passed in 10 min only.

$$\text{So, Min PHF}_{10 \text{ min}} = \frac{V}{\left(\frac{60}{10}\right) \times V} = \frac{1}{6} = 0.167$$

2. (d)

Radius of Horizontal curve = 75 m

$$\begin{aligned} \text{Grade compensation} &= \frac{30 + R}{R} \leq \frac{75}{R} \\ &= \frac{30 + 75}{75} = 1.4 \end{aligned}$$

$$\text{and } \frac{75}{R} = \frac{75}{75} = 1\%$$

So, Grade compensation = 1%

∴ Compensated gradient

$$\begin{aligned} &= \text{Ruling gradient} - \text{grade compensation} \\ &= 6\% - 1\% = 5\% \end{aligned}$$

3. (a)

$$\text{Capacity} = \frac{1000V}{S}$$

$$\begin{aligned} S &= 0.2V + L \\ &= 0.2 \times 60 + 6 = 18 \text{ m} \end{aligned}$$

$$\text{Capacity} = \frac{1000 \times 60}{18} \times 2 = 6666.7 \simeq 6700$$

The closest answer is 6800.

4. (b)

$$R = \frac{v^2}{127(e+f)} = \frac{362}{127(0.1+0.15)} = 40.82$$

5. (c)

$$\begin{aligned} \text{CBR}_{2.5} &= \frac{\text{Load sustained by specimen at 2.5 mm penetration}}{\text{Load sustained by standard aggregate at 2.5 mm penetration}} \times 100 \\ &= \frac{60}{1370} \times 100 = 4.379 \simeq 4.38\% \end{aligned}$$

$$\begin{aligned} \text{CBR}_{5.0} &= \frac{\text{Load sustained by specimen at 5.0 mm penetration}}{\text{Load sustained by standard aggregate at 5.0 mm penetration}} \times 100 \\ &= \frac{80}{2055} \times 100 = 3.89\% \end{aligned}$$

6. (a)

Given:

$$n_a = 21, n_w = 350, V = 70 \text{ kmph}$$

$$T_w = T_a = \frac{8}{70} \text{ hr}$$

$$q = \frac{n_a + n_w}{T_a + T_w} = \frac{350 + 21}{\left(\frac{8}{70} + \frac{8}{70}\right)} \simeq 1624 \text{ vehicles/hr}$$

7. (c)

The theoretical specific gravity of the mix is given by,

$$G_t = \frac{100}{\frac{50}{2.56} + \frac{38.20}{2.65} + \frac{4.70}{2.70} + \frac{7.10}{1.10}} = 2.37$$

8. (d)

$$\text{Hourly expansion factor} = \frac{25000}{5000} = 5$$

9. (c)

Curve (a) or (d) → For flow value

Curve (b) → Percent voids filled with bitumen (VFB)

Curve (c) → Percent voids in total mix

10. (c)

$$\frac{\Delta_{flexible}}{\Delta_{rigid}} = \frac{1.5}{1.18} = 1.27$$

11. (d)

12. (d)

13. (a)

Numbers of points of conflict:

Crossing : 4

Merging : 8

Weaving : 12

14. (a)

Assume S is less than L

$$L = \frac{NS^2}{4.4}$$

$$N_1 = \frac{1}{50} = 0.02$$

$$N_2 = -\frac{1}{40} = -0.025$$

$$\therefore N = N_1 - N_2 = 0.02 - (-0.025) = 0.045$$

$$\therefore L = \frac{0.045 \times 180 \times 180}{4.4} = 331.36 \text{ m}$$

Hence assumption is correct.

Equation of parabola is,

$$\begin{aligned} y &= \frac{Nx^2}{2L} \\ &= \frac{0.045x^2}{2 \times 331.36} = 6.79 \times 10^{-5} x^2 \end{aligned}$$

15. (d)

Given: $V = 60 \text{ kmph} = \frac{60 \times 1000}{60 \times 60} = 16.67 \text{ m/s}$

$$\begin{aligned} \text{SSD} &= Vt + \frac{V^2}{2g\left(f + \frac{n\%}{100}\right)} \\ &= 16.67 \times 2.5 + \frac{(16.67)^2}{2 \times 9.81 \times (0.36 + 0.02)} \\ &= 78.948 \simeq 78.95 \text{ m} \\ \therefore \text{ISD} &= 2 \times \text{SSD} = 157.9 \text{ m} \end{aligned}$$

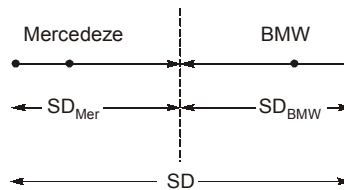
16. (a)

$$\begin{aligned} L &= 2 \left[\frac{NV^3}{c} \right]^{1/2} \\ &= 0.38 [NV^3]^{1/2} \quad [\because c = 0.6 \text{ m/s}^3] \end{aligned}$$

$$N = -\frac{1}{45} - \left(\frac{1}{60} \right) = \frac{-7}{180}$$

$$\begin{aligned} \therefore L &= 0.38 \left[\frac{7}{180} \times 75^3 \right]^{1/2} \\ &= 48.67 \text{ m} \end{aligned}$$

17. (b)



$$\text{Stopping distance} = SD_{\text{Mer}} + SD_{\text{BMW}}$$

$$\begin{aligned} \text{Stopping distance for Mercedes} &= (0.278 V_{\text{Mer}})t + \frac{V_{\text{Mer}}^2}{254f} \\ &= 0.278 \times 220 \times 2 + \frac{220^2}{254 \times 0.7 \times 0.8} \\ &= 462.59 \text{ m} \end{aligned}$$

$$\text{Stopping distance for BMW} = 0.278 \times 180 \times 3 + \frac{180^2}{254 \times 0.7 \times 0.75} = 393.09 \text{ m}$$

$$\begin{aligned} \therefore \text{Total stopping distance required} &= 462.59 + 393.09 \\ &= 855.68 \text{ m} \end{aligned}$$

18. (c)

Speed Range	Mean	Frequency	$V_i q_i$	$\frac{q_i}{V_i}$
2 – 6	4	2	8	0.5
7 – 11	9	5	45	0.555
12 – 14	13	1	13	0.0769
15 – 19	17	8	136	0.47
Total		16	202	1.60

$$\text{Time mean speed, } V_t = \frac{\sum q_i V_i}{\sum q_i} = \frac{202}{16} = 12.625$$

$$\text{Space mean speed, } V_s = \frac{\sum q_i}{\sum \left(\frac{q_i}{V_i} \right)} = \frac{16}{1.6} = 10$$

$$\text{Ratio of time mean speed to space mean speed} = \frac{12.625}{10} \approx 1.26$$

19. (d)

$$\begin{aligned} V_v &= \frac{G_t - G_m}{G_t} \times 100 \\ &= \frac{2.464 - 2.318}{2.464} \times 100 = 5.925\% \end{aligned}$$

$$\text{Voids filled with bitumen, } V_b = G_m \times \frac{W(\%)}{G_b}$$

$$= 2.318 \times \frac{5.5}{1.12} = 11.383\%$$

$$\begin{aligned} \text{VMA} &= V_v + V_b \\ &= 5.925 + 11.383 = 17.308\% \end{aligned}$$

$$\begin{aligned} \therefore \text{VFB} &= \frac{V_b (\%)}{\text{VMA}} \times 100 \\ &= \frac{11.383}{17.308} \times 100 = 65.767\% \approx 65.77\% \end{aligned}$$

20. (a)

21. (d)

$$\text{Deflection, } \Delta = \frac{1.5pa}{E_s}$$

where, p = Contact pressure due to wheel load = 0.5 MPa, a = Radius of contact area

$$\begin{aligned} \text{Now, Contact area} &= \frac{\text{Wheel load}}{\text{Type pressure}} \\ &= \frac{50 \times 10^3 \text{ N}}{0.5} = 100 \times 10^3 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} \text{But Area} &= \pi \times a^2 \\ &= 100 \times 10^3 \end{aligned}$$

$$\therefore a = \sqrt{\frac{100 \times 10^3}{\pi}} = 178.41 \text{ mm}$$

$$E_s = 20 \text{ MPa}$$

$$\text{So, } \Delta = \frac{1.5 \times 0.5 \times 178.41}{20} = 6.69 \text{ mm}$$

22. (b)

$$\begin{aligned} \text{Traffic flow, } q &= uk \\ &= 60k - 0.75k^2 \end{aligned}$$

For maximum q ,

$$\frac{dq}{dk} = 0$$

$$\Rightarrow \frac{dq}{dk} = 60 - 1.5k = 0$$

$$\Rightarrow k = 40$$

$$\therefore u = 60 - 0.75 \times 40 = 30 \text{ km/h}$$

$$\therefore q = 30 \times 40 = 1200 \text{ veh/h}$$

23. (c)

$$\text{The capacity of rotary, } Q_p = \frac{280w \left(1 + \frac{e}{w}\right) \left(1 - \frac{p}{3}\right)}{\left(1 + \frac{w}{L}\right)}$$

$$w = 15 \text{ m, } p = 0.6, L = 75 \text{ m, } e = 5 \text{ m}$$

$$\Rightarrow Q_p = \frac{280 \times 15 \times \left(1 + \frac{5}{15}\right) \left(1 - \frac{0.60}{3}\right)}{\left(1 + \frac{15}{75}\right)} = 3733.33 \approx 3733 \text{ PCU/hr}$$

24. (a)

$$P(n) = \frac{(\lambda t)^n e^{-\lambda t}}{n!}$$

where,

 λ = Average rate of arrival, t = time interval n = Number of vehicles $P(n)$, is the probability of n events (vehicle arrival) in some time interval t .

$$\Rightarrow \lambda = \frac{200}{30 \times 60} = \frac{1}{9} \text{ vehicle/sec}$$

Now probability that the headway (into arrival time) greater than or equal to 5 sec is

$$P(n=0) = \frac{\left(\frac{1}{9} \times 5\right)^0 \times e^{-\left(\frac{1}{9} \times 5\right)}}{0!} = e^{-\frac{5}{9}} = 0.573$$

25. (b)

We know that,

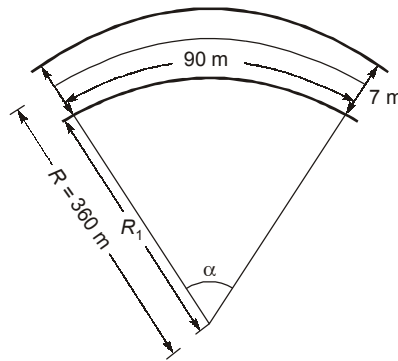
$$C_0 = \frac{1.5L + 5}{1 - Y}$$

$$L = 9 \text{ sec}$$

$$Y = \frac{800}{2000} + \frac{900}{3000} = 0.7$$

$$C_0 = \frac{1.5 \times 9 + 5}{1 - 0.7} = 61.67 \approx 62 \text{ sec}$$

26. (b)



\Rightarrow We know that, $R_1\alpha = \text{SSD}$,

$$R_1 = 360 - \frac{3.5}{2} = 358.25\text{ m}$$

$$358.25 \times \alpha = 90$$

$$\alpha = 0.251\text{ radian} = 14.39^\circ$$

For set back distance

We know,

$$m = R - (R - d)\cos\frac{\alpha}{2}$$

$$= 360 - (360 - 1.75)\cos\left(\frac{14.39}{2}\right)$$

$$m = 4.57\text{ m}$$

Set back distance from center line of inner lane = $m - d = 4.57 - 1.75 = 2.82\text{ m}$

27. (c)

$$N_s = \frac{A \times 365 \times \left(\left(1 + \frac{r}{100} \right)^n - 1 \right) \times L.D.F \times V.D.F}{\left(\frac{r}{100} \right)}$$

$$= \frac{2000 \times 365 \times [(1.1)^{15} - 1] \times 0.75 \times 2.8}{\left(\frac{10}{100} \right)}$$

$$= 48.70\text{ msa}$$

28. (c)

During stoppage number of vehicle passes = 40

Stoppage time = 10 min

$$\text{Stream flow } (q) = 40 \times \frac{60}{10} = 240 \text{ veh/hr}$$

$$\begin{aligned} \eta_y &= \text{Number of overtaking vehicles} - \text{Number of overtaken vehicle} \\ &= 70 - 25 = 45 \end{aligned}$$

$$\Rightarrow \text{We know that, } \eta_y = q[t_w - \bar{t}]$$

$$\bar{t} = t_w - \frac{\eta_y}{q} = \left(\frac{25}{60}\right) - \left(\frac{45}{240}\right) = 0.2292 \text{ hr}$$

$$\text{Average speed} = \frac{12}{0.2292} \approx 52.36 \text{ km/hr}$$

29. (a)

Axle load (kN)	Mean Axle load (kN)	Load factor $(L/80)^4$	Repetition (N)	
20-30	25	9.536×10^{-3}	2000	19.072
30-40	35	0.0366	1000	36.60
40-50	45	0.100	1500	150
50-60	55	0.223	500	111.5
			$\Sigma N_s = 5000$	317.172

$$\text{Vehicle damage factor} = \frac{317.172}{2100} \approx 0.15$$

30. (b)

Maximum expansion allowed,

$$\delta = \frac{3}{2} = 1.5 \text{ cm} = 1.5 \times 10^{-2} \text{ m}$$

$$\Delta T = T_2 - T_1 = 30^\circ\text{C}$$

Coefficient of thermal expansion,

$$\alpha = 10 \times 10^{-6}/^\circ\text{C}$$

 \therefore Spacing of expansion joint, L

$$= \frac{\delta}{\alpha(T_2 - T_1)} = \frac{1.5 \times 10^{-2}}{10 \times 10^{-6} \times 30} = 50 \text{ m}$$

