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# **FLUID MECHANICS**

## CIVIL ENGINEERING

Date of Test: 21/09/2022

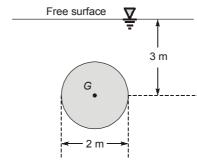
# ANSWER KEY >

1.	(a)	7.	(b)	13.	(d)	19.	(b)	25.	(b)
2.	(d)	8.	(c)	14.	(a)	20.	(b)	26.	(c)
3.	(d)	9.	(b)	15.	(a)	21.	(d)	27.	(a)
4.	(b)	10.	(b)	16.	(b)	22.	(a)	28.	(b)
5.	(c)	11.	(a)	17.	(b)	23.	(c)	29.	(d)
6.	(b)	12.	(d)	18.	(b)	24.	(c)	30.	(b)

### INCIDE ERSU

### **DETAILED EXPLANATIONS**

#### 2. (d)



$$d = 2 \,\mathrm{m}$$

$$A = \frac{\pi}{4} \times 2^2 = 3.14 \text{ m}^2$$

$$\bar{h} = 3.0 \, \text{m}$$

Total pressure, 
$$F = \rho g A \overline{h}$$

$$= 1000 \times 9.81 \times 3.14 \times 3 = 92.41 \text{ kN}$$

#### 4. (b)

For laminar boundary layer over a flat plate

$$\tau = C_f \frac{\rho v^2}{2}$$

where

$$C_f = \frac{0.664}{\sqrt{R_{ex}}}$$

$$\Rightarrow \qquad \qquad \tau \, \propto \, \frac{1}{\sqrt{x}}$$

$$\Rightarrow \frac{\tau_1}{\tau_2} = \sqrt{15}$$

#### 5. (c)

For pipes in series

$$\frac{L}{a^5} = \frac{L_1}{d_1^5} + \frac{L_2}{d_2^5} + \frac{L_3}{d_3^5}$$

$$\Rightarrow \frac{6000}{a^5} = \frac{1000}{(0.2)^5} + \frac{2000}{(0.4)^5} + \frac{3000}{(0.6)^5}$$

$$\Rightarrow d = 0.282 \,\text{m} \quad \text{or} \quad d = 282 \,\text{mm}$$

#### 6. (b)

In this case Froude model law is applicable

Given: 
$$L_r = \frac{1}{15}$$
  
 $V_r = \sqrt{L_r}$ 

7

$$\therefore$$
 Force ratio,  $F_r = L_r^3$ 

$$\therefore$$
 Force on prototype =  $0.8 \times (15)^3 = 2700$  Newton

#### 7. (b)

$$a = \frac{\pi}{4}d^2 = \frac{\pi}{4} \times (0.070)^2 = 0.00385 \,\mathrm{m}^2$$

Velocity of jet, V = 10 m/s

The force exerted by the jet of water on a stationary vertical plate is given as,

$$F = \rho a V^2 = 1000 \times 0.00385 \times 10^2$$
  
= 385 N

#### 12. (d)

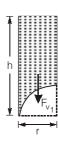
Horizontal component  $(F_v) = \rho g \overline{h} A$ 

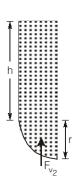
where A is projected area of gate

$$F_x = \rho gh \times 2rw = 2\rho ghrw$$

$$F_{H}$$
 = Pressure × Projector arc of gate

= 
$$\rho gh \times (2r \times w) = 2\rho ghrw$$





$$F_{v_1} = \rho g \forall = \rho g \left( h \times r - \frac{\pi r^2}{4} \right) w$$

$$F_{v_2} = \rho g \forall = \rho g \left( h \times r + \frac{\pi r^2}{4} \right) w$$

(downwards)

Vertical component,

$$(F_y) = (F_{v_2} - F_{v_1})$$

$$= \rho gw \left[ 2 \times \frac{\pi r^2}{4} \right]$$

$$= \frac{\pi \rho gw r^2}{2}$$

Positive sign indicates that net vertical component of hydrostatic force is in upward direction.

#### 13. (d)

As the pipe is rigid, velocity of pressure wave,

$$C = \sqrt{\frac{K}{\rho}} = \left(\frac{2.2 \times 10^9}{998}\right)^{1/2} = 1484.7 \text{ m/s}$$

Critical time

$$T_o = \frac{2L}{C} = \frac{2 \times 3500}{1484.7} = 4.715 \text{ s}$$

As T = time of closure = 4.0s, the closure is rapid

Hence the water hammer pressure,

$$P_h = \rho CV_0 = 998 \times 1484.7 \times 0.8 = 1.185 \text{ MPa}$$



#### 14. (a)

Reynold number at the trailing end,

$$Re_L = \frac{UL}{v} = \frac{1 \times 3}{10^{-5}} = 3 \times 10^5 < 5 \times 10^5$$

⇒ Boundary layer is laminar over the entire length of plate.

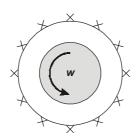
Thus, thickness of boundary layer at the end of the plate from Blasius's solution is,

$$\delta = \frac{5 \times L}{\sqrt{Re_L}} = \frac{5 \times 3}{\sqrt{3 \times 10^5}} = 0.0274 \,\text{m}$$

$$\simeq 27.4 \, \text{mm}$$

#### 15. (a)

The torque is transmitted through the fluid layers to the outer cylinder.



Tangential velocity of the inner cylinder

$$= \omega r = \frac{2\pi n}{60} \times r = 0.2 \times \frac{2\pi 60}{60} = 1.257 \text{ m/s}$$

For small space between the cylinders, the velocity profile may be assumed to be linear, and thus

$$\frac{du}{dy} = \frac{1.257}{(205 - 200) \times 10^{-3}} = 251.4 \text{ per sec}$$

Torque applied = Torque resisted

$$\Rightarrow$$
 0.98 =  $\tau \times \text{Area} \times \text{Lever arm}$ 

$$\Rightarrow \qquad 0.98 = \tau \times (2\pi \times 0.20 \times 0.30) \times 0.20$$

$$\Rightarrow$$
  $\tau = 12.998 \simeq 13 \text{ N/m}^2$ 

$$\mu = \frac{\tau}{du/dy} = \frac{13}{251.4} = 0.052 \,\text{N-s/m}^2$$

#### 16. (b)

$$V = \frac{\partial \psi}{\partial x} = \frac{\partial (6x^2 - y^3)}{\partial x} = 12x$$

$$u = \frac{-\partial \psi}{\partial y} = \frac{-\partial (6x^2 - y^3)}{\partial y} = 3y^2$$

At point (2, 1),

$$v = 24 \text{ units}$$

$$u = 3 \text{ units}$$

.. The total velocity is the vector sum of two components i.e.,

$$\Rightarrow$$
  $V = \sqrt{u^2 + v^2} = \sqrt{3^2 + 24^2} = 24.19 \text{ units}$ 

#### 17. (b)

Circulation,

$$\Gamma = \text{Vorticity} \times \text{Area}$$

$$= \left(\frac{\partial V}{\partial x} - \frac{\partial U}{\partial y}\right) \times \text{Area}$$

$$= (2 - 5) \times \pi \times 1^{2}$$

$$= -3\pi \text{ units}$$

#### 18. (b)

From Bernoulli's equation, we have

$$\frac{\rho_{1}}{\rho g} + \frac{V_{1}^{2}}{2g} = \frac{\rho_{2}}{\rho g} + \frac{V_{2}^{2}}{2g} + h_{c}$$

$$\Rightarrow \frac{14 \times 10^{4}}{1000 \times 9.81} + \frac{1.6^{2}}{2 \times 9.81} = \frac{12 \times 10^{4}}{1000 \times 9.81} + \frac{6.4^{2}}{2 \times 9.81} + h_{c}$$

$$\Rightarrow h_{c} = 0.0816 \text{ m}$$
Now,
$$h_{c} = \frac{V_{2}^{2}}{2g} \left[ \frac{1}{C_{c}} - 1 \right]^{2}$$

$$\Rightarrow 0.0816 = \frac{V_{2}^{2}}{2 \times 9.81} \left[ \frac{1}{C_{c}} - 1 \right]^{2}$$

$$\Rightarrow 0.0816 = \frac{6.4^{2}}{2 \times 9.81} \left[ \frac{1}{C_{c}} - 1 \right]^{2}$$

$$\Rightarrow C_{c} = 0.835 \simeq 0.84$$

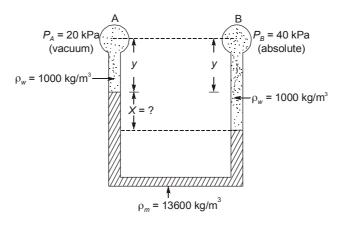
#### 19. (b)

$$C_D = \frac{24}{Re}$$

$$Re = \frac{1000 \times 0.1 \times 0.1 \times 10^{-2}}{1} = 0.1$$

$$C_D = \frac{24}{0.1} = 240$$

#### 22. (a)



Equating pressure in two limbs.

$$P_{A} + \dot{P}_{w} g y + P_{m} g x = P_{B} + \dot{P}_{w} g y + P_{w} g x$$

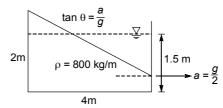
$$P_{A} = -20 \text{ kPa} \qquad (given)$$

$$P_{B} = 40 \text{ kPa} \qquad (given)$$

$$(\rho_{m} - \rho_{w}) \times g \times x = (40 + 20) \text{ kPa}$$

$$x = \frac{60 \times 1000}{12600 \times 9.81} = 0.485 \text{ m}$$

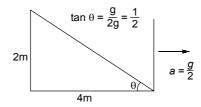
23. (c)



⇒ Surface of liquid will make angle with horizontal = a/g

$$\Rightarrow \tan \theta = \frac{a}{g} = \frac{g/2}{g} = \frac{1}{2}$$

$$\Rightarrow \qquad \frac{y}{4} = \frac{1}{2}$$



$$\Rightarrow \qquad \qquad y = 2 \text{ m}$$

$$\Rightarrow \qquad \qquad \text{Initial volume} = 1.5 \times 4 \times 2.5 = 15 \text{ m}^3$$

$$\Rightarrow \qquad \text{Finally retained volume } = \frac{1}{2} \times 2 \times 4 \times 2.5 = 10 \text{ m}^3$$

$$\Rightarrow$$
 Spilled out volume =  $15 - 10 = 5 \text{ m}^3$ 

24. (c)

$$\tau_r = -\frac{\partial \rho}{\partial x} \frac{r}{2}$$

$$\tau_{\text{well}} = -\frac{\partial \rho}{\partial x} \frac{R}{R}$$

Since flow is taking place under same pressure gradient, therefore

$$\frac{\tau_r}{\tau_w} = \frac{r}{R} \Rightarrow \tau_r = \tau_w \frac{r}{R} = 72 \times \frac{50}{90} = 40 \text{ N/m}^2$$

25. (b)

Reynold's number must be same for model and prototype

$$\frac{V_p D_p}{v_p} = \frac{V_m D_m}{v_m}$$

$$\Rightarrow \frac{\rho_p V_p D_p}{\mu_p} = \frac{\rho_m V_m D_m}{\mu_m}$$

$$\Rightarrow V_m = V_p \cdot \frac{D_p}{D_m} \cdot \frac{\rho_p}{\rho_m} \frac{\mu_m}{\mu_p}$$

$$= 1.7099 \times \frac{2.2}{0.20} \times \frac{0.9}{1} \times \frac{0.01}{0.03} = 5.64 \text{ m/s}$$

$$V_{p} = \frac{Q_{p}}{A_{p}} = \frac{6.5}{\frac{\pi}{4}(2.2)^{2}} = 1.7099 \text{ m/s}$$

Discharge in the model =  $5.64 \times \frac{\pi}{4} \times 0.20^2 = 0.18 \text{ m}^3/\text{s}$ 

#### 26. (c)

$$F = \rho A V^{2}$$

$$V = C_{V} \sqrt{2gH}$$

$$= 0.98 \sqrt{2 \times 10 \times 50} = 30.99 \text{ m/s}$$

$$F = 1000 \times \frac{\pi}{4} \times (0.030)^{2} \times 30.99^{2}$$

$$= 678.87 \text{ N} \simeq 0.68 \text{ kN}$$

#### 27.

Maximum head available at the outlet of the pipe =  $H - h_f$ 

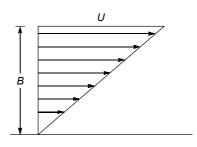
$$= H - \frac{H}{3} = \frac{2H}{3} = \frac{2 \times 400}{3} = 266.67 \text{ m}$$

∴ Maximum power available =  $\rho Qg(H - h_f)$ 

$$= \frac{1000 \times 9.81 \times 0.2 \times 266.67}{1000} \text{ kW}$$
$$= 523.2 \text{ kW}$$

28. (b)

 $\Rightarrow$ 



$$q = \frac{U}{}$$

29. (d)

Mean velocity, 
$$V = \frac{Q}{A} = \frac{300 \times 10^{-3}}{\left(\frac{\pi}{4}\right) \times (0.3)^2} = 4.244 \text{ m/s}$$

Shear velocity, 
$$u_* = v\sqrt{\frac{f}{8}} = 4.244 \times \sqrt{\frac{0.0097}{8}} = 0.1478 \text{ m/s}$$

Maximum velocity  $u_m$  is related to the mean velocity in both smooth and rough turbulent flows as

$$\frac{u_m - V}{u_*} = 3.75$$

$$u_m = 3.75 \ U_* + V = 3.75 \times 0.1478 + 4.244$$
$$= 4.798 \ \text{m/s} \approx 4.8 \ \text{m/s}$$

30. (b)

Discharge through a venturimeter is given by

$$Q = \frac{a_1 a_2 \sqrt{2gh}}{\sqrt{a_2^2 - a_1^2}}$$

where, 
$$a_2 = \frac{\pi}{4} \times 0.3^2 = 0.0707 \,\text{m}^2$$

$$a_1 = \frac{\pi}{4} \times 0.15^2 = 0.0177 \,\mathrm{m}^2$$

and 
$$Q = 120 l/s = 0.12 m^3/s$$

$$\Rightarrow \qquad 0.12 = \frac{0.0177 \times 0.0707 \times \sqrt{2 \times 9.81 \times h}}{\sqrt{(0.0707)^2 - (0.0177)^2}}$$

$$\Rightarrow$$
  $h = 2.196 \,\mathrm{m}$ 

Also, head, 
$$h = x \left( \frac{S_h}{S_l} - 1 \right)$$

where,  $S_h$  and  $S_l$  are specific gravities of heavier and lighter manometric fluids.

$$S_h = S_{Hg} = 13.6$$

$$S_l = S_{water} = 1$$

$$\Rightarrow 2.196 = x (13.6 - 1)$$

$$\Rightarrow$$
  $x = 0.1743 \,\text{m} = 17.43 \,\text{cm}$