

CLASS TEST

S.No. : 02 SK1_CE_B_010619

Irrigation Engineering



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CLASS TEST 2019-2020

CIVIL ENGINEERING

Irrigation Engineering

Date of Test : 01/06/2019

Answer Key

- | | | | | |
|--------|---------|---------|---------|---------|
| 1. (c) | 7. (c) | 13. (b) | 19. (d) | 25. (c) |
| 2. (a) | 8. (b) | 14. (c) | 20. (b) | 26. (d) |
| 3. (a) | 9. (a) | 15. (a) | 21. (b) | 27. (c) |
| 4. (c) | 10. (c) | 16. (a) | 22. (c) | 28. (c) |
| 5. (d) | 11. (b) | 17. (d) | 23. (b) | 29. (b) |
| 6. (a) | 12. (d) | 18. (d) | 24. (b) | 30. (c) |

DETAILED EXPLANATIONS

1. (c)

$$\begin{aligned} \text{Sodium adsorption ratio (SAR)} &= \frac{[\text{Na}^+]}{\sqrt{\frac{[\text{Ca}^{2+}] + [\text{Mg}^{2+}]}{2}}} = \frac{20}{\sqrt{\frac{10+5}{2}}} \\ &= 7.30 \end{aligned}$$

2. (a)

As per Kennedy's

Critical velocity,

$$V_0 = 0.55 m y^{0.64}$$

 $m =$ critical velocity ratio = 0.90 $y =$ depth of flow = 1.2 m \therefore

$$V_0 = 0.55 \times 0.9 \times (1.2)^{0.64} = 0.556 \text{ m/sec}$$

Also, discharge through channel,

$$Q = 4 \text{ m}^3/\text{sec}$$

 \therefore Required cross-section area of channel

$$A = \frac{Q}{V_0} = \frac{4}{0.556} \simeq 7.20 \text{ m}^2$$

4. (c)

To prevent scouring

$$\gamma_w RS < 0.056 \gamma_w d(G_s - 1)$$

where $d =$ particle size ($> 6 \text{ mm}$)

$$G_s = 2.65$$

 \Rightarrow

$$RS \leq \frac{d}{11}$$

$$R \leq \frac{d}{11S}$$

$$R_{\max} = \frac{d}{11S}$$

Given, $d = 6 \text{ cm}$ ($> 6 \text{ mm}$) ok, $S = 0.01$

$$R_{\max} = \frac{6 \times 10^{-2}}{0.01 \times 11} = 0.5454 \text{ m} = 54.54 \text{ cm}$$

5. (d)

$$Q = 5 \text{ m}^3/\text{s}$$

$$\text{Area, } A = 20 \text{ ha}$$

$$\text{Time} = 5 \text{ hrs}$$

$$\text{Water stored in root zone} = 0.4 \text{ m}$$

∴ Water application efficiency

$$\eta_a = \frac{\text{Water stored in root zone}}{\text{Water supplied to field}} \times 100$$

$$= \frac{0.4 \times 20 \times 10^4}{5 \times 5 \times 3600} \times 100 = 88.88\%$$

6. (a)

Given,

Field capacity = 18.3%

Root zone depth, $d = 1.2$ m

Existing moisture content, $W = \frac{W_w}{W_s} = \frac{153 - 138}{138} \times 100 = 10.87\%$

∴ Depth of water required to be applied to bring the moisture upto its field capacity.

$$d_w = \frac{\gamma_d}{\gamma_w} \cdot d \cdot (FC - mc)$$

$$= 1.25 \times 1.2 \times \left(\frac{18.3 - 10.87}{100} \right) = 0.111 \text{ m} \simeq 111 \text{ mm}$$

10. (c)

Mean depth, $D = \frac{2 + 1.9 + 1.8 + 1.6 + 1.5}{5} = 1.76$ m

∴ Value of deviation from mean are (2 - 1.76), (1.9 - 1.76), (1.8 - 1.76), (1.6 - 1.76), (1.5 - 1.76)
= 0.24, 0.14, 0.04, -0.16, -0.26

Average of absolute deviations

$$d = \frac{0.24 + 0.14 + 0.04 + 0.16 + 0.26}{5} = 0.168 \text{ m}$$

Water distribution efficiency, $\eta_d = \left(1 - \frac{d}{D} \right) \times 100 = \left(1 - \frac{0.168}{1.76} \right) \times 100 = 90.45\%$

11. (b)

As per Lane's, he suggested a weighting factor of 1/3 to horizontal creep and 1 for vertical creep.

∴ Creep length, $L = 2d_1 + L/3 + 2d_2$
= $2 \times 8 + 30/3 + 2 \times 10 = 46$ m

∴ Hydraulic gradient $i = \frac{H}{L} = \frac{6}{46} = 0.13$

12. (d)

Net vertical force,

$$\Sigma V = W - U = 1036 - 674 = 362 \text{ kN}$$

Net horizontal force,

$$\Sigma H = \text{Water force}$$

$$= \frac{1}{2} \cdot \gamma_w H^2 = \frac{1}{2} \times 9.81 \times (10)^2 = 490.5 \text{ kN}$$

∴ Shear friction factor,
$$\text{SFF} = \frac{\mu \cdot \Sigma V + B \cdot q}{\Sigma H}$$

B = width of the base of foundation = 8.25 m; μ = 0.75; q = shear strength at the joint = 1400 kN/m²

$$\text{SFF} = \frac{0.75 \times 362 + 8.25 \times 1400}{490.5} = 24.10$$

13. (b)

Intensity of irrigation for kharif = 100 – 65 = 35%

Intensity of irrigation for rabi = 100 – 50 = 50%

∴ Annual intensity of irrigation = sum of seasonal intensity of irrigation in a year
 = 35% + 50% = 85%.

14. (c)

Given,

$$Q = 50 \text{ m}^3/\text{sec}$$

Silt factor,

$$f = 1.1$$

As per Lacey's

∴ Velocity,

$$V = \left(\frac{Qf^2}{140} \right)^{1/6} = \left(\frac{50 \times (1.1)^2}{140} \right)^{1/6} = 0.869 \text{ m/sec}$$

Bed slope,

$$S = \frac{f^{5/3}}{3340 Q^{1/6}} = \frac{(1.1)^{5/3}}{3340(50)^{1/6}} = 0.0001828$$

$$S = \frac{1}{5469}$$

15. (a)

Leaching requirement,

$$LR = \frac{(EC)_i}{(EC)_d}$$

$(EC)_i$ = Electrical conductivity of irrigation water
 = 1.2 milli mho/cm

$(EC)_d$ = Electrical conductivity of drained water
 = $2 \times (EC)_e$

$(EC)_e$ = Electrical conductivity of saturated soil extract
 = 10 milli mho/cm

∴

$$LR = \frac{(EC)_i}{2 \cdot (EC)_e} \times 100 = \frac{1.2}{2 \times 10} \times 100 = 6\%$$

Also,

$$LR = \frac{D_d}{D_i}$$

$$D_i = C_u + D_d$$

C_u = consumptive use = 80 mm; D_d = depth of water drained out; D_i = depth of water applied for irrigation

$$LR = \frac{D_i - D_u}{D_i}$$

$$\frac{6}{100} = \frac{D_i - 80}{D_i}$$

$$6 D_i = 100 D_i - 80 \times 100$$

$$94 D_i = 8000$$

$$D_i = 85.1 \text{ mm}$$

16. (a)

Classification	E.C in μ Mho/cm	Exchangable sodium percentage, ESP(%)	pH
1. Saline or white alkali soil	> 4000	< 15	≤ 8.5
2. Alkaline or black alkali soil	< 4000	> 15	8.5 – 10
3. Soline-Alkali soil	> 4000	> 15	< 8.5

17. (d)

For non-scouring, $d \leq 11 RS$

$$R_{\max} = \frac{d}{11S}$$

$$V = \frac{1}{n} \cdot R^{2/3} \cdot S^{1/2}$$

$$n = \frac{1}{24} d^{1/6}$$

$$V = \frac{24}{d^{1/6}} R^{2/3} \cdot S^{1/2}$$

$$V_{\max} = \frac{24}{d^{1/6}} (R_{\max})^{2/3} \cdot S^{1/2}$$

$$= \frac{24}{d^{1/6}} \left(\frac{d}{11S} \right)^{2/3} \cdot S^{1/2} = \frac{24}{(11)^{2/3}} \cdot d^{2/3-1/2} \cdot S^{1/2-2/3}$$

$$V_{\max} = 4.85 d^{1/2} \cdot S^{-1/6} \quad (\text{where, } d \text{ is in m})$$

If d is in cm, then,

$$V_{\max} = 4.85 \left(\frac{d}{100} \right)^{1/2} \cdot S^{-1/6}$$

$$= 0.485 d^{1/2} \cdot S^{-1/6} \quad (d \text{ is in cm})$$

$$V_{\max} \simeq 0.48 d^{1/2} \cdot S^{-1/6}$$

18. (d)

As per Blaney-Criddle formula

$$C_u \text{ or PET} = \sum \frac{kp}{40} [1.8t + 32]$$

$$= \frac{0.65 \times 9.3}{40} [1.8 \times 28 + 32] + \frac{0.72 \times 10.6}{40} [1.8 \times 25 + 32]$$

$$= 27.14 \text{ cm}$$

In above equation, k = consumptive use coefficient/crop factor; p = monthly %age of annual day light hours; t = temperature ($^{\circ}\text{C}$).

19. (d)

$$G_E = \frac{H}{d} \frac{1}{\pi \sqrt{\lambda}}$$

where,

$$H = \text{Total head} = 1.5 \text{ m}$$

$$\lambda = \frac{1 + \sqrt{1 + \alpha^2}}{2}$$

$$d = \text{Depth of d/s cutoff} = 2 \text{ m}$$

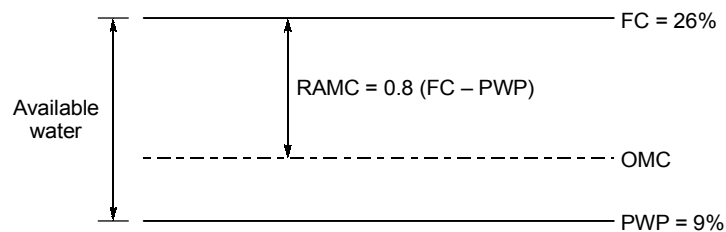
$$\alpha = \frac{b}{d} = \frac{13}{2} = 6.5$$

$$\lambda = \frac{1 + \sqrt{1 + 6.5^2}}{2} = 3.79$$

$$\therefore G_E = \frac{1.5}{2} \cdot \frac{1}{\pi \sqrt{3.79}}$$

$$\Rightarrow G_E = 0.123 \approx 0.12$$

20. (b)



$$d = \text{depth of root zone} = 75 \text{ cm}$$

$$C_u \text{ per day} = 1.58 \text{ cm/day}$$

$$\therefore \text{RAMC} = \text{Readily available depth of water}$$

$$= \frac{\gamma_d}{\gamma_w} \cdot d \cdot (FC - OMC) = \frac{1.4}{1} \times 0.75 \times 0.8 \times (0.26 - 0.09)$$

$$= 0.1428 \text{ m}$$

$$\therefore \text{Frequency of irrigation, FOI} = \frac{\text{RAMC}}{C_u} = \frac{(0.1428 \times 100) \text{ cm}}{1.58 \text{ cm/day}} = 9.03 \approx 9 \text{ days}$$

21. (b)

$$D = 0.19 \text{ m}$$

$$B = 14 \text{ days}$$

$$\text{Outlet factor, } D = \frac{8.64 B}{\Delta} = \frac{8.64 \times 14}{0.19} = 636.6 \text{ ha/m}^3/\text{sec} \approx 637 \text{ ha/m}^3/\text{sec}$$

25. (c)

Area, $A = 0.04 \text{ ha} = 0.04 \times 10^4 = 400 \text{ m}^2$
 Depth of flow, $y = 10 \text{ cm} = 0.10 \text{ m}$
 Infiltration rate, $f = 5 \text{ cm/hr}$
 $Q = 0.02 \text{ m}^3/\text{sec}$

time taken to irrigate the crop

$$t = \frac{y}{f} \cdot 2.303 \cdot \log_{10} \left(\frac{Q}{Q - fA} \right)$$

$$= \frac{10}{5} \times 60 \times 2.303 \cdot \log_{10} \left(\frac{0.02}{0.02 - \frac{5 \times 10^{-2}}{3600} \times 400} \right) \text{ min}$$

$$= 39.05 \approx 39 \text{ min}$$

27. (c)

90% of initial capacity = $0.9 \times 4 \times 10^6 = 36 \times 10^5 \text{ m}^3$
 Volume of sediment deposited annually till 90% of initial capacity is filled
 = (Annual sediment \times trap efficiency in flow)
 = $4 \times 10^6 \times 0.9 = 36 \times 10^3 \text{ m}^3$
 \therefore Number of years during which 90% of initial capacity shall be filled is given by

$$\text{Probable life of reservoir} = \frac{36 \times 10^5}{36 \times 10^3} = 100 \text{ years}$$

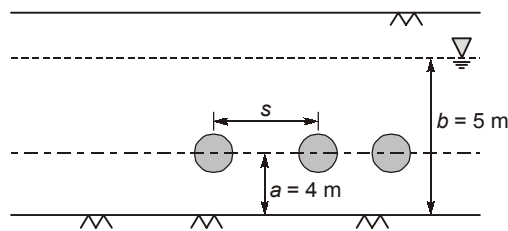
28. (c)

$$B = \frac{H}{\sqrt{S_c - C}} \quad (C = 0; \text{ when uplift is ignored})$$

$$B = \frac{H}{\sqrt{S_c}}$$

$$H = 35 \times \sqrt{2.65} = 56.97 \text{ m} \approx 57 \text{ m}$$

29. (b)



Spacing of tile drain,

$$s = \frac{4k}{q} (b^2 - a^2) = \frac{4 \times 10^{-6}}{1.8 \times 10^{-6}} \times (5^2 - 4^2) = 20 \text{ m}$$

30. (c)

$$\text{Sinuosity of meander} = \frac{\text{Curve length of meander}}{\text{Straight length of meander}}$$

